FOOTINGS, WALLS, & BOND BEAMS

1. CONCRETE MASONRY UNITS SHALL BE HOLLOW OR SOLID UNIT MASONRY IN ACCORDANCE WITH ASTM C 90 OR C 145 AND SHALL HAVE MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.
2. MORTAR SHALL BE EITHER TYPE M OR S IN ACCORDANCE WITH ASTM C 270.
3. GROUT SHALL HAVE A MAXIMUM COARSE AGGREGATE SIZE OF 3/8 INCH PLACED AT A 8 TO 11 INCH SLUMP AND HAVE MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS WHEN TESTED IN ACCORDANCE WITH ASTM C 1019, OR SHALL BE IN ACCORDANCE WITH ASTM C 476.
4. CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS.
5. REINFORCING STEEL SHALL BE MINIMUM GRADE 40 AND IDENTIFIED IN ACCORDANCE WITH ASTM A 615, A 616, A 617, OR A 706.
6. JOINT REINFORCEMENT, ANCHORS, TIES, AND WIRE FABRIC SHALL CONFORM TO THE FOLLOWING STANDARDS: ASTM A 82 FOR JOINT REINFORCEMENT AND WIRE ANCHORS AND TIES. ASTM A 36 FOR PLATE, HEADED AND BENT BAR ANCHORS. ASTM A 366 FOR SHEET METAL ANCHORS AND TIES.
7. METAL ACCESSORIES FOR USE IN EXTERIOR WALL CONSTRUCTION AND NOT DIRECTLY EXPOSED TO THE WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A 153, CLASS B-2. METAL ACCESSORIES FOR USE IN INTERIOR WALL CONSTRUCTION SHALL BE MILL GALVANIZED IN ACCORDANCE WITH ASTM A 641, CLASS 1.
8. ALL MORTAR JOINTS FOR HOLLOW UNIT MASONRY SHALL EXTEND THE FULL WIDTH OF FACE SHELLS.
9. MORTAR JOINTS FOR SOLID MASONRY SHALL BE FULL HEAD AND BED JOINTS. BED JOINTS SHALL BE 3/8 INCH (Ñ 1/8 INCH) THICK. HEAD JOINTS SHALL BE 3/8 INCH (+ 3/8 INCH OR - 1/4 INCH) THICK.
10. THE BED JOINT OF THE STARTING COURSE PLACED OVER FOOTINGS SHALL BE PERMITTED TO VARY IN THICKNESS FROM A MINIMUM OF 1/4 INCH TO A MAXIMUM OF 3/4 INCH.
11. MASONRY WALLS SHALL BE RUNNING BOND OR STACK BOND CONSTRUCTION.
12. WHEN MASONRY UNITS ARE LAID IN STACK BOND OR RUNNING BOND, A 9-GAGE (MINIMUM) HORIZONTAL JOINT REINFORCEMENT, IN ADDITION TO REQUIRED VERTICAL REINFORCEMENT, SHALL BE PLACED IN BED JOINTS AT NOT MORE THAN 16 INCHES ON CENTER.
13. LONGITUDINAL WIRES OF JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR OR GROUT WITH MINIMUM COVER OF 5/8 INCH WHEN EXPOSED TO EARTH OR WEATHER AND 1/2 INCH WHEN NOT EXPOSED TO EARTH OR WEATHER.
14. REINFORCING STEEL SHALL BE NO. 5 BARS.
15. SPLICES SHALL BE LAP SPLICES.
16. NONCONTACT LAP SPLICES MAY BE USED PROVIDED REINFORCING BARS ARE NOT SPACED FARTHER APART THAN 5 INCHES.
17. SPLICE LENGTHS SHALL BE MINIMUM OF 25 INCHES.
18. REINFORCEMENT MAY BE BENT IN THE SHOP OR IN THE FIELD PROVIDED: ALL REINFORCEMENT SHALL BE BENT COLD, AND THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX BAR DIAMETERS, AND REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT, EXCEPT WHERE BENDING IS NECESSARY TO ALIGN DOWEL BARS WITH A VERTICAL CELL, BARS PARTIALLY EMBEDDED IN CONCRETE SHALL BE PERMITTED TO BE BENT AT SLOPE OF NOT MORE THAN 1 INCH OF HORIZONTAL DISPLACEMENT TO 6 INCHES OF VERTICAL BAR LENGTH.
19. FOR FOUNDATIONS MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE 3 INCHES.
20. FOOTINGS FOR STEMWALL FOUNDATIONS SHALL BE A MINIMUM OF 16" THICK BY 24" WIDE, WITH THREE (3) #5 REINFORCING BARS.
21. FOOTING FOR MONOLITHIC SLAB ON GRADE FOUNDATIONS SHALL BE A MINIMUM OF 16" THICK BY 24" WIDE, WITH THREE (3) #5 REINFORCING BARS.
22. IN NARROW FOOTING WHERE INSUFFICIENT WIDTH IS AVAILABLE TO ACCOMMODATE A STANDARD 90 DEGREE HOOK AND PROVIDE THE REQUIRED CONCRETE COVER, THE HOOK SHALL BE ROTATED IN THE HORIZONTAL DIRECTION UNTIL THE REQUIRED CONCRETE COVER IS ACHIEVED.
23. FOR CAST-IN-PLACE TIE BEAMS THE MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 1 1/2 INCHES.
24. REINFORCEMENT BARS EMBEDDED IN GROUTED MASONRY CELLS SHALL HAVE A MINIMUM CLEAR DISTANCE OF 1/2 INCH BETWEEN REINFORCING BARS AND ANY FACE OF A CELL.
25. REINFORCING BARS USED IN MASONRY WALLS SHALL HAVE A MASONRY COVER (INCLUDING GROUT) OR NOT LESS THAN 2 INCHES.
26. CLEANOUT OPENINGS SHALL BE PROVIDED FOR CELLS CONTAINING SPLICED REINFORCEMENT WHEN THE GROUT POUR EXCEEDS 5 FEET IN HEIGHT.
27. WHERE CLEANOUT OPENINGS ARE REQUIRED, AN OPENING SHALL BE PROVIDED IN THE BOTTOM COURSE OF THE MASONRY CELL TO BE FILLED.
28. CLEANOUT OPENINGS SHALL HAVE MINIMUM AREA OF 12 SQUARE INCHES AND A MINIMUM OPENING DIMENSION OF 3 INCHES.
29. MASONRY PROTRUSIONS EXTENDING 1/2 INCH OR MORE INTO CELLS OR CAVITIES TO BE GROUTED SHALL BE REMOVED FOR GROUT POURS OVER 5 FT.
30. SPACES TO BE GROUTED SHALL BE FREE OF MORTAR DROPPINGS, DEBRIS, LOOSE AGGREGATES, AND ANY MATERIAL DELETERIOUS TO MASONRY GROUT. 31. A SOIL OR WASTE PIPE OF A BUILDING DRAIN PASSING UNDER A FOOTING OR THROUGH A FOUNDATION WALL SHALL BE PROVIDED WITH A RELIEVING ARCH, OR THERE SHALL BE BUILT INTO THE MASONRY WALL AN IRON PIPE SLEEVE TWO PIPE SIZES GREATER THAN THE PIPE PASSING THROUGH.
32. THE TOP AND BOTTOM OF ALL FOOTINGS SHALL BE LEVEL. THE BOTTOM OF ALL FOOTINGS, EXCEPT MONOLITHIC SLAB-ON-GRADE INTERIOR FOOTINGS, SHALL BE A MINIMUM OF 12" BELOW FINISHED GROUND LINE.
33. THE OUTER BAR OF FOUNDATION STEEL SHALL BE CONTINUOUS AROUND CORNERS USING CORNER BARS OR BY BENDING THE BAR IN ACCORDANCE WITH NOTES HEREIN, IN BOTH CASES, THE MINIMUM BAR LAP SHALL BE 25 INCHES.
34. FOUNDATION STEMWALLS SHALL BE 10 INCHES THICK, AND SHALL HAVE SAME VERTICAL REINFORCING AS THE WALL ABOVE.
35. FOOTING DOWELS BARS SHALL BE PROVIDED FOR ALL REQUIRED VERTICAL WALL REINFORCEMENT IN THE FOLLOWING LOCATION: AT ALL CORNERS, AT EACH SIDE OF EACH OPENING, AT ALL OTHER REQUIRED VERTICAL WALL REINFORCEMENT AT ALL HIP GIRDER BEARING POINTS.
36. FOOTING DOWEL BARS AT EACH LOCATION SHALL BE SAME SIZE AND QUANTITY AS THE VERTICAL WALL REINFORCEMENT ABOVE.
37. ALL FOOTING DOWEL BARS SHALL HAVE A STANDARD 90 DEGREE HOOK AND SHALL BE EMBEDDED A MIN. OF 6" INTO FOOTINGS.
38. CONCRETE SLAB-ON-GRADE SHALL BE CAST IN PLACE AND SHALL BE 4 INCHES THICK MINIMUM. CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF NOT LESS THAN 2500 PSI AT 28 DAYS.
39. THE MINIMUM THICKNESS OF EXTERIOR MASONRY WALLS SHALL BE 8 INCHES.
40. A REINFORCED TIE BEAM SHALL BE PROVIDED AT THE TOP OF EACH EXTERIOR WALL.
41. TIE BEAMS SHALL BE 8"X 16" HIGH CAST-IN-PLACE CONCRETE.
42. TIE BEAM REINFORCEMENT SHALL BE FOUR NO. 5 BARS EXCEPT WHERE NOTED.
43. REINFORCEMENT SHALL BE LOCATED IN THE TOP AND BOTTOM OF 16 INCH BOND BEAMS.
44. REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNERS. SEE STRUCTURAL DETAILS.
45. WHERE MORE THAN ONE BAR IS REQUIRED, ONLY ONE OF THE BARS MUST BE CONTINUOUS AROUND CORNERS.
46. FOR VERTICAL REINFORCEMENT ONE NO. 5 BAR IN A GROUTED CELL SHALL BE PROVIDED IN EACH CORNER, INCLUDING INTERIOR CORNERS AND CORNERS CREATED BY CHANGES IN WALL DIRECTION BY OFFSETTING OF WALLS SUCH AS AT PROJECTED BAYS AND INSET PORCHES.
47. FOR VERTICAL REINFORCEMENT ONE NO. 5 BAR SHALL BE PROVIDED ON EACH SIDE OF OPENINGS.
48. IN ADDITION TO VERTICAL REINFORCEMENT REQUIRED AT CORNERS, AT OPENINGS, AND AT HIP GIRDER BEARING POINTS, VERTICAL REINFORCEMENT CONSISTING OF ONE NO. 5 BAR SHALL BE PROVIDED EVERY 6 FEET ON CENTER MAXIMUM.
49. ALL VERTICAL WALL REINFORCEMENT SHALL BE TERMINATED IN THE BOND BEAM AT THE ROOF LEVEL WITH A STANDARD HOOK. THE HOOK MAY BE FORMED BY BENDING THE VERTICAL WALL REINFORCEMENT IN ACCORDANCE WITH NOTES HEREIN OR BY LAP SPLICING TO A STANDARD HOOK. THE HOOK SHALL EXTEND TO THE UPPER MOST HORIZONTAL REINFORCEMENT OF THE BOND BEAM AND SHALL BE EMBEDDED A MINIMUM OF 6 INCHES INTO THE BOND BEAM,. SEE STANDARD DETAILS.
51. CONTINUOUS TIE BEAMS ACTING AS LINTELS SHALL BE 16 INCHES HIGH HAVE TOP AND BOTTOM REINFORCEMENT CONTINUOUS OVER THE WALL AND OPENING. THE AMOUNT OF REINFORCEMENT SHALL BE AS FOLLOWS; UNLESS ADDITIONAL REINFORCEMENT IS REQUIRED DUE TO LARGE BEARING LOADS (I.E. HIP GIRDER BEARING POINTS).
52. TIE BEAMS SHALL HAVE TOP AND BOTTOM REINFORCEMENT CONTINUOUS OVER OPENINGS.
53. TIE BEAMS WHICH HAVE ADDITIONAL REINFORCEMENT OVER OPENINGS WHICH IS IN ADDITION TO THAT REQUIRED OVER THE WALL SHALL EXTEND PAST THE OPENING A MINIMUM OF 24".

A. OPENINGS UP TO 6'-0" SHALL HAVE FOUR (4) #5 BARS.
B. OPENINGS FROM 6'-1" TO 8"-0" SHALL HAVE FIVE (5) BARS. TWO (2) TOP AND THREE (3) BOTTOM.
C. OPENINGS FROM 8'-1" TO 12'-0" SHALL HAVE SIX (6) BARS. TWO (2) TOP AND FOUR (4) BOTTOM.
D. OPENINGS FROM 12'-1" TO 16'-0" SHALL HAVE SEVEN (7) #5 BARS. TWO (2) TOP AND FIVE 5) BOTTOM, WITH #3 STIRRUPS AT 8" O.C., OR TWO (2) #5 TOP AND FOUR (4) #7 BOTTOM, WITH STIRRUPS AT 8" O.C.

54. STEMWALL FOUNDATION HEIGHT SHALL NOT EXCEED 3'-0" FROM FINISHED GRADE TO TOP OF MASONRY.
55. COLUMNS SHALL BE CONSTRUCTED OF STANDARD MASONRY UNITS.
56. MAXIMUM COLUMN HEIGHT (TO THE TOP OF THE BOND BEAM) SHALL BE 10 FT.
57. COLUMNS SHALL CONTAIN A MINIMUM OF FOUR VERTICAL BARS, ONE IN EACH CORNER.
    A. VERTICAL COLUMN REINFORCEMENT SHALL BE FOUR NO. 3 BARS FOR 8X8 INCH COLUMNS AND FOUR NO. 5 BARS FOR ALL OTHER COLUMN SIZES.
    B. CLEARANCE FROM THE VERTICAL BAR TO THE MASONRY FACE SHELL SHALL BE 1/2 INCH. MINIMUM COVER FOR CAST IN PLACE COLUMNS SHALL BE 1 1/2 INCHES OVER THE COLUMN TIES.
58. CONNECTION OF COLUMNS TO THE FOUNDATION BELOW AND TO THE BOND BEAM AT THE TOP SHALL BE AS FOLLOWS:
    A. 8X8 INCH COLUMN: ONE NO. 5 STANDARD 90 DEGREE HOOK INTO THE SUPPORT AT THE BOTTOM AND INTO THE BOND BEAM AT THE TOP.
    B. 8X16 INCH COLUMN: TWO NO. 5 STANDARD 90 DEGREE HOOKS (ONE IN EACH CELL) BOTH AT THE BOTTOM AND AT THE TOP.
    C. 12X12 INCH COLUMN AND 16X16 INCH COLUMN: BOTTOM: FOUR NO. 5 STANDARD 90 DEGREE HOOKS (ONE AT EACH VERTICAL BAR) EXTENDING FROM THE FOUNDATION AND SPLICED WITH THE VERTICAL COLUMN REINFORCEMENT; TOP: FOR CORNER COLUMNS, THREE NO. 5 STANDARD 90 DEGREE HOOKS INTO THE BOND BEAM, MINIMUM, EACH SPLICED TO A VERTICAL COLUMN BAR. FOR COLUMN LOCATED OTHER THAN AT A CORNER, TWO NO. 5 STANDARD 90 DEGREE HOOK INTO THE BOND BEAM SHALL BE SPLICED TO SEPARATE VERTICAL COLUMN BARS.
59. LATERAL TIES OF A MINIMUM 1/4 INCH DIAMETER SHALL BE USED TO ENCLOSE VERTICAL COLUMN REINFORCEMENT AS FOLLOWS:
    A. MAXIMUM VERTICAL SPACING OF LATERAL TIES SHALL BE 12".
    B. LATERAL TIES MAY BE PLACED IN MORTAR JOINTS (PROVIDED THEY ARE NO LARGER THAN 1/4 INCH DIAMETER).
    C. THE BOTTOM LATERAL TIES SHALL BE LOCATED VERTICAL NOT MORE THAN 1/2 A LATERAL TIE SPACING ABOVE THE TOP OF THE FOOTING.
    D. THE TOP LATERAL TIE SHALL NOT BE MORE THAN 1/2 A LATERAL TIE SPACING BELOW THE LOWEST HORIZONTAL REINFORCEMENT IN THE BEAM ABOVE.
60. A STEMWALL FLOATING SLAB FOUNDATION SHALL NOT BE PERMITTED UNDER THE UNENCLOSED WALLS OF A BUILDING.
61. ALL CONCRETE IS TO MIXED, TRANSPORTED, AND PLACED IN ACCORDANCE WITH THE LATEST ACI SPECIFICATIONS AND RECOMMENDATIONS.
62. FOUNDATIONS HAVE BEEN DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF, AND THE EXISTING SOIL BEING A GRANULAR MATERIAL SHOULD POOR SOIL CONDITIONS BE FOUND IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER PRIOR TO COMMENCING.
63. PROVIDE GRANULAR FILL, CLAY MATERIALS ARE UNACCEPTABLE. EXISTING SOIL UNDER FOOTING AND SLABS SHALL BE CLEAN AND COMPACTED TO 95% OF AASHTO T-99.
64. FILL SHALL BE PLACED AND COMPACTED IN ONE FOOT LIFTS.
65. A CONCRETE SLAB-ON-GRADE USED IN CONJUNCTION WITH EXTERIOR STEMWALL FOUNDATIONS SHALL HAVE 6X6 NO. 10 WELDED WIRE FABRIC AT MID -HEIGHT OR SYNTHETIC FIBER REINFORCEMENT IN THE SLAB AND THE SLAB SHALL BE KEYED INTO OR TIED TO THE FOUNDATION.
66. A DOUBLE LAYER OF WELDED WIRE FABRIC SHALL BE PROVIDED AROUND THE PERIMETER OF THE CONCRETE SLAB A DISTANCE OF 3 FT. FROM THE EDGE.
67. WELDED WIRE FABRIC SHALL CONFIRM TO ASTM A-185 AND FREE OF OIL AND RUST. IT SHALL BE INSTALLED IN LENGTHS AS LONG AS POSSIBLE AND LAPPED A MINIMUM OF SIX INCHES. 68. PROVIDE (1) #5 ELECTRICAL GROUND TO FOUNDATION STEEL.
69. A 6 MIL MINIMIUM POLYETHYLENE DAMPROOFING VAPOR BARRIER SHALL BE PROVIDED.
70. FILL SHALL BE TERMITE TREATED AND A "CERTIFICATE FOR TERMITE TREATMENT" IS REQUIRED ON THE PERMIT BOARD PURSUANT TO FBC SEC. 105.10.
71. ALL FOOTINGS SHALL BE A MINIMUM OF 12" BELOW FINISHED GRADE.
72. THE TOP OF SLAB SHALL BE A MINIMUM OF 8" ABOVE FINISHED GRADE FOR WOOD FRAME CONSTRUCTION.
73. THE TOP OF SLAB SHALL BE A MINIMUM OF 4" ABOVE FINISHED GRADE FOR MASONRY VENEER AND A MINIMUM OF 6" ELSEWHERE.

CONCRETE

SHALL MEET ALL THE REQUIREMENTS OF ACI 301-84 WITH TYPE I OR II CEMENT. MINIMUM 28 DAY STRENGTH - 3000PSI UNLESS NOTED OTHERWISE.
CURBS, GUTTERS, WALKS, EXTERIOR SLABS, AND TRUCK RAMP: 4000PSI
INTERIOR SLAB AT OFFICES AND SALES AREA: 3000 PSI
NO ADMIXTURES WITHOUT APPROVAL. ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED. CONCRETE SHALL NOT BE IN CONTACT WITH ALUMINIUM.
MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, EXCEPT SLABS ON GRADE NEED BE VIBRATED ONLY AROUND EMBEDDED ITEMS. MAXIMUM SLUMP - 4". DO NOT TAMP SLABS. USE ROLLER BUG, VIBRATING SCREED OR BULL FLOAT TO FINISH. SEE SPECIFICATIONS FOR CURING. CAST SLABS ON GRADE IN ALTERNATE SECTIONS, UNLESS PERMANENT FORMS ARE USED. WAIT 48 HOURS BETWEEN ALL ADJACENT CONCRETE CASTINGS. REVIBRATE TOPS OF COLUMNS AND CAISSONS SOON AFTER PLACING CONCRETE, TO CLOSE PLASTIC SHRINKAGE CRACKS.
MINIMUM STRENGTH FOR REMOVAL OF FORMS AND SHORING SHALL BE 70% OF SPECIFIED STRENGTH AT 28 DAYS.
USE OF FLY ASH IS NOT PERMITTED FOR ANY CONCRETE USED IN SLABS.

MASONRY

REINFORCED MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH = 1500 PSI.
MASONRY UNITS SHALL CONFORM TO ASTM C90.
MORTAR SHALL CONFORM TO ASTM C270, TYPE S, 1800 PSI.
GROUT SHALL CONFORM TO ASTM C476, 2000 PSI.
MECHANICALLY VIBRATE GROUT IN VERTICAL SPACES IMMEDIATELY AFTER POURING AND AGAIN ABOUT 5 MINUTES LATER. MAXIMUM GROUT LIFT WITHOUT CLEANOUTS 60". STAY EACH END OF EACH VERTICAL REBAR USING SINGLE WIRE AND LOOP TYPE TIES. MAXIMUM VERTICAL SPACING OF TIES 8"-0". SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINTS.
LOCATE AT 32 FEET MAXIMUM O.C. U.N.O., BUT NOT LESS THAN 2'-0" FROM A BEARING PLATE OR FROM A JAMB OF AN OPENING WIDER THAN 4'-0", U.N.O.
BACKFILL AGAINST MASONRY WALLS IS NOT PERMITTED UNTIL SUFFICIENT LATERAL SUPPORT IS PROVIDED.
MINIMUM VERTICAL REINFORCING TO BE # 5 @ 48" O.C.
PROVIDE (2) NO. 3 BARS IN BED JOINT IMMEDIATELY ABOVE AND BELOW OPENINGS IN BEARING WALLS. EXTENDING 24" BEYOND EACH JAMB, FOR ALL OPENINGS LESS THAN 2'-0" IN SIZE. PROVIDE HORIZONTAL JOINT REINFORCEMENT ARE ANCHORED TO FLOORS. PLACE BARS AT ROOF AND FLOOR LINES CONTINUOUS THROUGH CONTROL JOINTS. (1) NO. 5 AT 16" O.C. IN ALL WALLS U.N.O.
MASONRY WALLS HAVE BEEN DESIGNED TO SPAN VERTICALLY AS SIMPLE SPANS FROM FLOOR TO ROOF AND ARE DEPENDENT UPON THE COMPLETED ROOF STRUCTURE, METAL ROOF DECK, AND COMPLETION OF ALL MASONRY WALLS FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING ALL NECESSARY BRACING AS REQUIRED FOR CONSTRUCTION LOADS. FOR STABILITY, AND RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THE ENTIRE STRUCTURE IS COMPLETE. THE SHORING SHALL NOT RELY ON ANY MOMENT RESISTANCE CAPACITY OF THE FOOTINGS.

CONCRETE FLOORS

1. CONCRETE FLOORS SHALL BE CAST IN PLACE.
2. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF NOT LESS THAN 3,000 PSI AT 28 DAYS.
3. THE TOP OF A MONOLITHIC SLAB-ON-GRADE SHALL BE AT LEAST 8 INCHES ABOVE FINISHED GRADE.
4. THE SLAB SHALL BE 4 INCHES THICK.
5. THE SLAB SHALL HAVE 6X6 NO. 10 WELDED WIRE FABRIC AT MID-HEIGHT OR SYNTHETIC FIBER REINFORCEMENT.
6. A DOUBLE LAYER OF WELDED WIRE FABRIC SHALL BE PROVIDED AROUND THE PERIMETER OF THE SLAB OF A DISTANCE OF 3 FT. FROM THE EDGE. SEE DETAIL SHEETS.
7. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND FREE OF OIL AND RUST. IT SHALL BE INSTALLED IN LENGTHS AS LONG AS POSSIBLE LAPPED A MINIMUM OF SIX INCHES.

STUCCO

APPLICATION OF STUCCO (PORTLAND CEMENT PLASTER) SHALL BE IN ACCORDANCE WITH ASTM C 926, APPLICATION OF PORTLAND_BASED STUCCO ON CMU AND/OR CONCRETE MUST BE A MINIMUM OF 1/2_INCH THICK FOR 2_COAT SYSTEM AND 5/8_INCH THICK FOR 3_COAT SYSTEM.
STUCCO ON METAL LATH MUST BE A MINIMUM OF 7/8_INCH THICK.

STRUCTURAL STEEL

ASTM A-36, EXCEPT AS FOLLOWS: PIPE STEEL: ASTM A-53 GRADE B, OR A-501. TUBE STEEL: ASTM A-500 GRADE B.
BOLTS AND PLAIN ANCHORS: ASTM A-307. LATEST AISC HANDBOOKS AND CODES APPLY.
MINIMUM EMBEDMENT OF ALL BOLTS IN GROUT OR CONCRETE SHALL BE 8" WITH A 3" HOOK.
WELDED ANCHORS AND SHEAR CONNECTORS SHALL BE ICOB APPROVED NELSON, KSM OR EQUAL.
ALL STEEL FABRICATION SHALL BE BY AN APPROVED FABRICATOR.
ALL STEEL BEARING TO BE ON A BOND BEAM, SOLID BLOCK CONCRETE OR STEEL DESIGNED FOR BEARING.

WELDING

ALL CONSTRUCTION AND TESTING PER AMERICAN WELDING SOCIETY CODES AND RECOMMENDATIONS. ALL WELDING SHALL BE DONE BY WELDERS HOLDING CURRENT VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN TYPE OF WELD CALLED FOR. WELDING RODS TO BE LOW HYDROGEN TYPE, E70.
ALL BUTT WELDED SPLICES IN MATERIAL THICKER THAN 5/16" SHALL BE INSPECTED BY AN INDEPENDENT TESTING LABORATORY, TO CERTIFY ALL SPLICES AS MEETING OR EXCEEDING STRENGTH OF MATERIAL SPLICED. TWO COPIES OF ALL TEST REPORTS AND A LETTER OF SUCH CERTIFICATION SHALL BE SUBMITTED TO THE ARCHITECT.
ALL WELDING OF REINFORCING SHALL CONFORM TO THE "STRUCTURAL WELDING CODES - REINFORCING STEEL", AWS D1.4-83. WELDS INDICATED MAY BE MADE IN SHOP OR FIELD WITH APPROVAL.

LIGHT GAUGE STUDS

1. ALL MEMBERS SHALL BE FORMED FROM CORROSION-RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTM A653-94 AND SHALL BE ZINC COATED MEETING ASTM A924. MINIMUM YIELD STRENGTH SHALL BE 33,000 PSI FOR ALL 20 GAUGE AND 18 GAUGE MEMBERS; AND 50,000 PSI FOR ALL 16 GAUGE, AND 12 GAUGE STUDS. PROPERTIES SHALL BE AS MANUFACTURED BY METAL STUD MANUFACTURERS ASSOCIATIONS PER I.C.B.O. NO. 4943 OR APPROVED EQUAL.
2. STUDS SHALL BE A MINIMUM 20 GAUGE "C" SHAPE, 1-5/8" WIDE, OF THE DEPTHS AND SPACING SHOWN ON DRAWINGS. TRACKS SHALL BE 18 GA., WITH A MINIMUM FLANGE OF 1-1/4".
3. BRIDGING SHALL BE COLD ROLLED CHANNEL, MINIMUM 1-1/2" DEEP WITH 9/16" FLANGE WIDTH. SPACE BRIDGING AT 4"-0" MAXIMUM O.C. VERTICALLY. DOUBLE UP STUDS AT ALL JAMBS. ALL CONNECTIONS SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
4. SPLICE IN HEAVY GAUGE FRAMING MEMBERS, OTHER THAN RUNNER TRACK SHALL NOT BE PERMITTED. ANCHOR RUNNER TRACK TO CONCRETE WITH 0.177 O X 1-1/2" HILTI FASTNERS OR EQUAL AT 16" O.C. MAX.

FASTENERS & CONNECTORS

1. APPROVED CONNECTORS, ANCHORS AND OTHER FASTENING DEVICES NOT INCLUDED IN THE 2020 7TH EDITION OF THE FLORIDA BUILDING CODE SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
2. WHERE FASTENERS ARE NOT OTHERWISE SPECIFIED FASTENERS SHALL BE PROVIDED IN ACCORDANCE WITH THE 2020 7TH EDITION FLORIDA BUILDING CODE. NAILS, SCREWS, OR BOLTS SHALL BE ABLE TO RESIST THE FORCES IN THIS CODE.
3. UNLESS OTHERWISE STATED, SIZES GIVEN FOR NAILS ARE COMMON WIRE NAILS. FOR EXAMPLE, 8D = 2 1/2 INCHES LONG X 0.131-INCH DIAMETER. SEE TABLE 12.3B, COLUMNS 2, 3, AND 4, IN THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION. METAL PLATES, CONNECTORS, SCREWS, BOLTS AND NAILS EXPOSED DIRECTLY TO THE WEATHER OR SUBJECT TO SALT CORROSION IN COASTAL AREAS, AS DETERMINED BY THE BUILDING OFFICIAL, SHALL BE STAINLESS STEEL, OR HOT DIPPED GALVANIZED AFTER THE FASTENER OR CONNECTOR IS FABRICATED TO FORM A ZINC COATING NOT LESS THAN 1 OZ PER SQ FT, OR HOT DIPPED GALVANIZED WITH A MINIMUM COATING OF 1.8 OZ PER SQ FT OF STEEL MEETING THE REQUIREMENTS OF ASTM A 90 TRIPLE SPOT TEST.

WOOD GENERAL

1. ALL WOOD CONSTRUCTION SHALL COMPLY WITH THE LATEST NFPA AND AITC SPECIFICATIONS AND RECOMMENDATIONS.
2. LUMBER STANDARD SHALL BE AMERICAN SOFTWOOD LUMBER STANDARD PS 20-70, S4S, 19% MOISTURE OR AS REQUIRED BY STRUCTURAL DESIGN.
3. STRUCTURAL LUMBER (ROOF BEAMS, HEADERS, COLUMNS, EXTERIOR WALL STUDS TO BE SOUTHERN PINE NO. 2 KD 15 WITH A FB=1,300 PSI E=1,600,000 PSI, AND FV = 95 PSI.
4. GLUE LAMINATED TIMBER SHALL CONFORM WITH ASTM D-3737 AND AITC 117. ROOF BEAMS SHALL BE DESIGNATED 24F-V1 OR 24F-E1.
5. PLYWOOD FOR SHEATHING SHALL BE APA RATED SHEATHING AS PER PLANS AND SHALL BEAR THE APA MARK.
6. WOOD IN CONTACT WITH CONCRETE, MASONRY AND/OR EXPOSED TO WEATHER SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109.

EXTERIOR WALL FRAMING

1. STUDS SHALL BE PLACED WITH THE WIDE FACE PERPENDICULAR TO THE WALL.
2.. HEADER BEAMS SHALL BE PROVIDED AND SIXED IN ACCORDANCE WITH THE 2020 7TH EDITION OF THE FLORIDA BUILDING CODE.
3. THE MINIMUM NUMBER OF HEADER STUDS SUPPORTING EACH END OF A HEADER BEAM SHALL BE 2.
4. THE MINIMUM NUMBER OF FULL-LENGTH WALL STUDS AT EACH END OF A HEADER BEAM SHALL BE 2 FOR OPENINGS OF 6 FEET OR LESS, AND 3 FEET FOR ALL OTHER OPENINGS.
5. UPLIFT CONNECTORS SHALL BE PROVIDED AT THE TOP AND BOTTOM OF CRIPPLE STUDS, OF HEADER STUDS, AND AT LEAST ONE WALL STUD AT EACH SIDE OF OPENING.

CONNECTIONS FOR EXTERIOR WALL FRAMING

1. FRAMING MEMBERS IN EXTERIOR WALL SYSTEMS SHALL BE FASTENED TOGETHER IN ACCORDANCE WITH THE 2020 7TH EDITION OF THE FLORIDA BUILDING CODE.
2. UPLIFT CONNECTORS SHALL BE PROVIDED TO RESIST THE UPLIFT LOADS.
3. UPLIFT LOAD RESISTANCE SHALL BE CONTINUOUS FROM ROOF TO FOUNDATION.
4. STUDS SHALL BE CONNECTED TO PLATES AND PLATES TO FLOOR FRAMING WITH CONNECTORS DESIGNED, RATED, AND APPROVED FOR EACH INDIVIDUAL LOCATION AND CONDITION. SEE WINDLOAD CONNECTORS SCHEDULE.
5. WHERE ANCHOR DOWN CONNECTORS OCCUR CONNECTORS REQUIRED FOR UPLIFT RESISTANCE MAY BE OMITTED.

BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB

Building Inspection Division - Jax., FL.

Sam West - September 16, 2022
Examiner Signature
License No.____________________

PLAN REVIEW AND ISSUING OF PERMIT DOES NOT RELIEVE CONTRACTOR OF COMPLYING WITH ALL CODES.

EXTERIOR WALLS

1. EXTERIOR WALL SEGMENTS SHALL NOT CONTAIN OPENINGS WHICH WHEN ADDED TOGETHER WILL EXCEED 144 SQ IN (1 SQ FT) IN ANY INDIVIDUAL SEGMENT.
2. EACH CORNER SHALL BE SHEATED FOR A LEAST 3 FEET AND MAY BE COUNTED AS A SHEARWALL SEGMENT.
3. MINIMUM LENGTH OF A SHEARWALL SEGMENT SHALL BE 2'-5". THE TOPS OF ALL SHEARWALL SEGMENTS IN ANY WALL SHALL BE CONNECTED BY DRAG STRUTS.
4. STUDS SHALL BE DOUBLED AT EACH END OF EACH SHEARWALL SEGMENT.
5. JOINTS SHALL BE LAP-SPLICED. WITHIN THE CENTER THIRD OF A WALL LENGTH, THE MINIMUM LAP SHALL BE 4 FEET. LAP SPLICES SHALL BE CONNECTED WITH 14 16D COMMON NAILS.
6. PROVIDE BRIDGING/BLOCKING AT 4'-0" O.C. AT EXTERIOR WALL STUDS.

PLYWOOD SHEATING USED FOR UPLIFT RESISTANCE

1. PANELS SHALL BE 5/8" EXPOSURE 1 SHEATHING GRADE PLYWOOD AND SHALL BE INSTALLED AS FOLLOWS:
   PANELS SHALL BE INSTALLED WITH FACE GRAIN PARALLEL TO STUDS. ALL HORIZONTAL JOINTS HALL OCCUR OVER FRAMING AND SHALL BE ATTACHED PER DETAIL SHEETS.
   FLATWISE BLOCKING SHALL BE USED AT ALL HORIZONTAL PANEL JOINTS.
   PANELS SHALL BE ATTACHED TO BOTTOM PLATES AND TOP MEMBER OF THE DOUBLE TOP PLATE.
   LOWEST PLATES SHALL BE ATTACHED TO FOUNDATION WITH BOLTS OR CONNECTORS OF SUFFICIENT CAPACITY TO RESIST THE UPLIFT FORCES DEVELOPED IN THE PLYWOOD SHEATHED WALLS.
   PANEL ATTACHMENT TO FRAMING SHALL BE AS ILLUSTRATED IN THE DETAIL SHEETS.
   WHERE WINDOWS AND DOORS INTERRUPT PLYWOOD SHEATHING, FRAMING ANCHORS OR CONNECTORS SHALL BE USED TO RESIST THE APPROPRIATE UPLIFT LOADS.

ANCHOR DOWN CONNECTORS

1. EXTERIOR WALLS REQUIRE ANCHOR DOWNS TO RESIST OVERTURNING MOMENT.
2. TWO STUDS AND ANCHOR DOWN ARE REQUIRED AT EACH END OF EACH SHEAR WALL SEGMENT.
3. THE ANCHOR DOWN SHALL BE FASTENED THROUGH THE DOUBLED STUDS AND TO THE CONSTRUCTION BELOW IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SEE WIND LOAD CONNECTORS SCHEDULE.

ROOF & TRUSS FRAMING SYSTEMS

1. ROOF TRUSS FRAMING SHALL BE SPACED A MAXIMUM OF 24" O.C.
2. TRUSSES SHALL BE NO. 2 SOUTHERN YELLOW PINE, UNLESS WHERE NOTED TO BE PRESSURE TREATED.
3. UPLIFT CONNECTORS SHALL BE PROVIDED AT TRUSS BEARING TO RESIST THE UPLIFT LOADS.
4. PROVIDE PRE-FABRICATED ROOF TRUSS LAYOUT DESIGNED BY A LICENSED FLORIDA ENGINEER.

ROOF SHEATHING

1. ROOF SHEETING SHALL BE 15/32 INCH EXPOSURE 1 C-D SHEETING GRADE PLYWOOD (WOOD STRUCTURAL PANELS), OR EQUIVALENT.
2. THE SHEETING SHALL BE INSTALLED IN ACCORDANCE WITH DETAIL SHEETS.
3. LONG DIMENSION SHALL BE PERPENDICULAR TO FRAMING AND END JOINTS SHALL BE STAGGERED.
4. SHEATHING SHALL BE FASTENED TO ROOF FRAMING WITH 8D RING-SHANK NAILS AT 6 INCHES ON CENTER AT EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE FRAMING. (PURSUANT TO THE 2020 7TH EDITION FLORIDA BUILDING CODE.)
   RING-SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:
   A. 0.113-INCH NOMINAL SHANK DIAMETER
   B. RING DIAMETER OF 0.012 OVER SHANK DIAMETER
   C. 16-20 RINGS PER INCH
   D. 0.280 INCH FULL ROUND HEAD DIAMETER
   E. $2\frac{3}{8}$"-INCH NAIL LENGTH

EXCEPTIONS:
WHERE ROOF DIAPHRAGM REQUIREMENTS MAY NECESSITATE A CLOSER FASTENER SPACING.

FASTENERS & CONNECTORS

1. APPROVED CONNECTORS, ANCHORS AND OTHER FASTENING DEVICES NOT INCLUDED IN THE 2020 7TH EDITION OF THE FLORIDA BUILDING CODE SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
2. WHERE FASTENERS ARE NOT OTHERWISE SPECIFIED FASTENERS SHALL BE PROVIDED IN ACCORDANCE WITH THE 2020 7TH EDITION OF THE FLORIDA BUILDING CODE. NAILS, SCREWS, OR BOLTS SHALL BE ABLE TO RESIST THE FORCES IN THIS CODE.
3. METAL PLATES, CONNECTORS, SCREWS, BOLTS AND NAILS EXPOSED DIRECTLY TO THE WEATHER OR SUBJECT TO SALT CORROSION IN COASTAL AREAS, AS DETERMINED BY THE BUILDING OFFICIAL, SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED AFTER THE FASTENER OR CONNECTOR IS FABRICATED TO FORM A ZINC COATING NOT LESS THAN 1 OZ PER SQ FT, OR HOT DIPPED GALVANIZED WITH A MINIMUM COATING OF1.8 OZ PER SQ FT OF STEEL MEETING THE REQUIREMENTS OF ASTM A 90 TRIPLE SPOT TEST.

GENERAL

1. LAND INVESTMENT SERVICES, INC. HAS NOT BEEN RETAINED TO PROVIDE, NOR ARE THEY RESPONSIBLE FOR, THE FIELD SUPERVISION, INSPECTION, OR CONSTRUCTION ADMINISTRATION OF THIS PROJECT.
2. THIS BUILDING/STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2020 FLORIDA BUILDING CODE, 7TH EDITION, FOR DESIGN PRESSURES GENERATED BY A THREE SECOND GUST DESIGN WIND VELOCITY OF 150 MPH, (116 MPH FASTEST MILE WIND VELOCITY). STRUCTURAL CALCULATIONS; INCLUDING GRAVITY LOADS, AS NECESSARY TO CONFIRM COMPLIANCE WITH THE 2020 7TH EDITION FLORIDA BUILDING CODE, HAVE BEEN PERFORMED.
3. THE OWNER, HIS AGENT, OR GENERAL CONTRACTOR IS RESPONSIBLE FOR FIELD SUPERVISION, CONSTRUCTION ADMINISTRATION, REVIEW AND APPROVAL OF ALL SHOP DRAWINGS, VERIFICATION ON-SITE OF ALL DIMENSIONS AND ELEVATIONS, AND STRICT COMPLIANCE WITH THESE CONSTRUCTION DOCUMENTS AS APPROVED BY LOCAL BUILDING DEPARTMENT, AND DESIGNED AND REVIEWED BY LIS ENGINEERING, LLC.
4. THESE PLANS ARE NOT INTENDED TO BE MASTERED. THE REPETITIVE USE OF THESE PLANS FOR PERMITTING IS NOT APPROVED.
5. ALL WINDOWS, DOORS, AND OTHER SUCH SYSTEMS, COMPONENTS AND CLADDING SHALL BE DESIGNED IN ACCORDANCE WITH THE 2020 FLORIDA BUILDING CODE 7TH EDITION FOR DESIGN PRESSURES SEC. 1609 GENERATED BY A THREE SECOND GUST DESIGN WIND VELOCITY OF 150 MPH, (116 MPH FASTEST MILE WIND VELOCITY), SEE "DESIGN PARAMETERS" FOR SPECIFIC PRESSURES.
6. CONTRACTOR SHALL VERIFY IN FIELD ALL EXISTING CONDITIONS SHOWN ON DRAWINGS.
7. CONTRACTOR SHALL NOTIFY THE OWNER IN WRITING PRIOR TO CONSTRUCTION OF ANY DISCREPANCY BETWEEN PLANS AND ON-SITE DIMENSIONS AND ELEVATIONS.
8. (1) ROOF DEAD LOAD = 30 PSF. DEAD LOAD AVAILABLE TO RESIST UPLIFT = 10 PSF
   (2) ROOF LIVE LOAD = 20 PSF (REDUCIBLE)
   (3) MECH- LOADS : SEE PLANS COORDINATE WITH CONTRACTOR.
9. PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR MECHANICAL, ELECTRICAL AND PLUMBING WITH APPROPRIATE TRADES, DRAWINGS AND SUB-CONTRACTORS PRIOR TO CONSTRUCTION.
10. THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, FOR SAFETY PRECAUTION AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTORS OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
11. UNLESS OTHERWISE NOTED, MINIMUM CONNECTION ELSEWHERE SHALL BE TWO 5/8" DIAM. BOLTS OR 1/4" FILLET WELD 4" LONG, USING 1/4" CONNECTION MATERIAL AND DETAILED TO MINIMIZE BENDING IN CONNECTION.
12. EPOXY ANCHORS SHALL BE 'SET' ADHESIVE BY SIMPSON ISBO #ER-5279 OR APPROVED EQUAL. TORS CONVENIENCE. HE SHALL BE RESPONSIBLE FOR ALL CHANGES NECESSARY IF HE CHOOSES AN OPTION AND HE SHALL COORDINATE ALL DETAILS.
14. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF FLORIDA.
15. UNLESS OTHERWISE NOTED, DETAILS ON STRUCTURAL DRAWINGS ARE TYPICAL AS INDICATED BY CUTS, REFERENCES OR TITLES.
16. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

LIS ARCHITECTURE ENGINEERING
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Digitally signed by ROBERT W CASE
Date: 2022.09.12 12:10:08 -04'00'

ROBERT W. CASE LICENSE No 44643 STATE OF FLORIDA PROFESSIONAL ENGINEER

ROBERT WAYNE CASE
FLORIDA PE. #44643

PLAN SET REVISIONS:

1	06/22/22 - BUILDING COMMENTS
2	09/12/22 - BUILDING COMMENTS

CONTRACT DATE:	12-18-2019
BUILDING TYPE:	Kb 30-19
PLAN VERSION:	
SITE NUMBER:	
ENTITY NUMBER:	
STORE NUMBER:	
LIS PROJECT;	2019-303

KFC

OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

Kb 30-19

STRUCTURAL NOTES

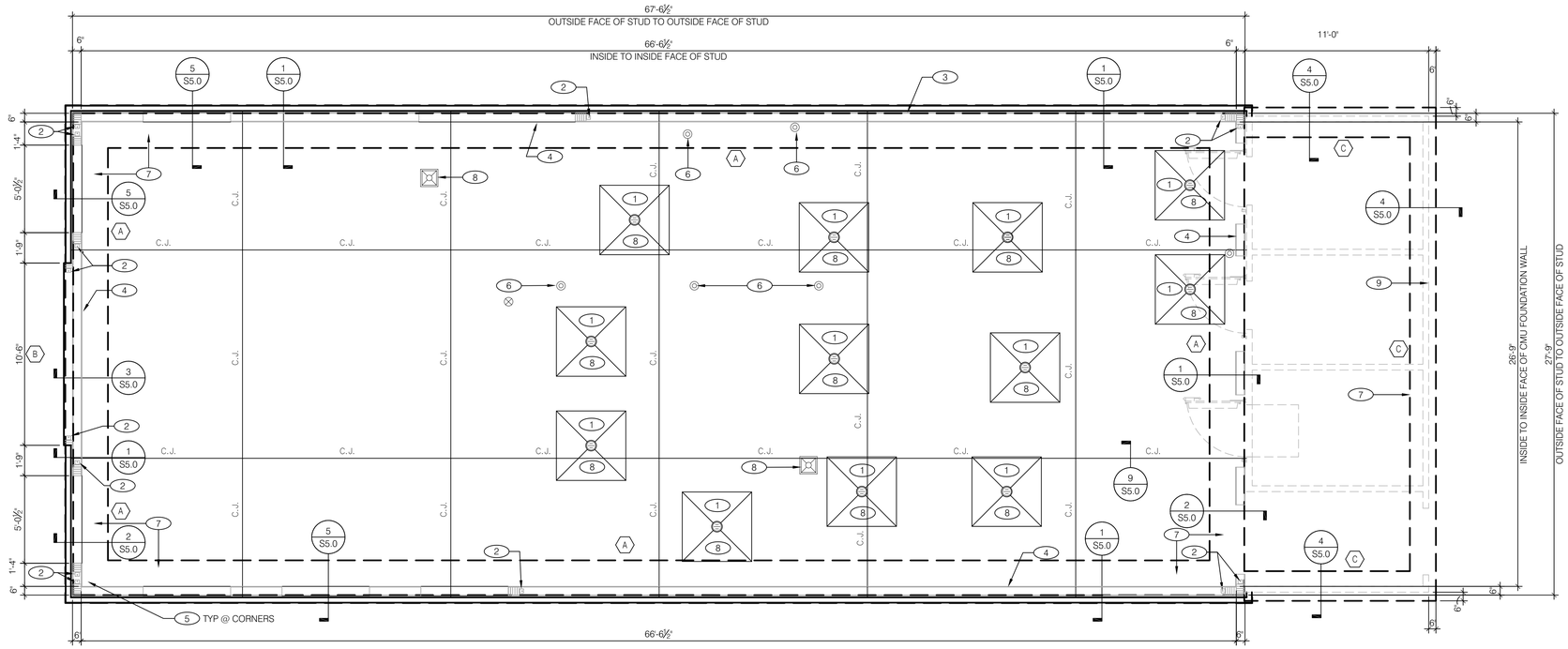
S1.0

B-22-511575.000
RCV: 9/12/2022 2:26 PM

PLOT DATE: 03/29/2022

(BUILDING LEFT)

(BUILDING FRONT)



(BUILDING REAR)

(BUILDING RIGHT)

CONCRETE FOOTING SCHEDULE

LABEL	TYPE	LENGTH	WIDTH	DEPTH	BOTTOM REINF. LONG WAY	BOTTOM REINF. SHORT WAY	TOP REINF. LONG WAY	TOP REINF. SHORT WAY	REMARKS
A	MONO	CONT.	2'-6"	1'-8"	(4) #5	N/A			(1) #5 TRANSVERSE @ 24" O.C. (INCLUDES BRICK LEDGE)
B	MONO	CONT.	2'-6"	1'-8"	(4) #5	N/A			(1) #5 TRANSVERSE @ 24" O.C.
C	MONO	CONT.	1'-6"	1'-8"	(2) #5	N/A			(1) #5 TRANSVERSE @ 24" O.C.

12 11 10 9 8 7 6 5 4 3 2 1

FOUNDATION PLAN 1/4"=1'-0" A

APPLICABLE CODES:
BUILDING CODES = 2020 FLORIDA BUILDING CODE 7TH EDITION
MECHANICAL CODE = 2020 FLORIDA BUILDING CODE, MECHANICAL 7TH EDITION
PLUMBING CODE = 2020 FLORIDA BUILDING CODE, PLUMBING 7TH EDITION
ELECTRICAL CODE = 2020 FLORIDA BUILDING CODE, ELECTRICAL 7TH EDITION
NFPA 70, NATIONAL ELECTRICAL CODE, 2018 EDITION
FIRE CODE = 2020 FLORIDA FIRE PREVENTION CODE 7TH EDITION
(NFPA 1 & NFPA 101), 2018
ACCESSIBILITY CODE = FLORIDA ACCESSIBILITY CODE FOR BUILDING
CONSTRUCTION, 2020 7TH EDITION
ENERGY CODE = 2020 FLORIDA ENERGY CODE, 7TH EDITION - CH13
(FLORIDA ENERGY EFFICIENCY FOR BUILDING CONSTRUCTION)

METHOD OF DESIGN:
DESIGNED PURSUANT TO 2020 FLORIDA BUILDING CODE,
SECTION 1609, AND ASCE 7-10

BASIC WIND SPEED:
[X] 150 MPH (ULTIMATE DESIGN) = 116 MPH (NOMINAL DESIGN)
[ ] 139 MPH (ULTIMATE DESIGN) = 108 MPH (NOMINAL DESIGN)

RISK CATEGORY:
[ ] 0.77 (BUILDING CATEGORY I)
[X] 1.00 (BUILDING CATEGORY II)

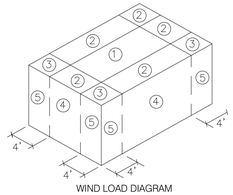
SURFACE ROUGHNESS:
[X] B [ ] C [ ] D

EXPOSURE CATEGORY:
[ ] B [X] C [ ] D

BUILDING CONSTRUCTION TYPE:
[ ] TYPE I-A [ ] TYPE II-B [ ] TYPE IV
[ ] TYPE I-B [ ] TYPE III-A [ ] TYPE V-A
[ ] TYPE II-A [ ] TYPE III-B [X] TYPE V-B

WINDBORNE DEBRIS REGION:
[ ] NO
[X] YES
[X] IMPACT RESISTANT GLAZING
[ ] IMPACT RESISTANT COVERING

INTERNAL PRESSURE COEFFICIENTS:
[ ] 0.00 (OPEN)
[X] +0.18, -0.18 (ENCLOSED)
[ ] +0.55, -0.55, (PARTIALLY ENCLOSED)



WIND LOAD DIAGRAM

COMPONENTS AND CLADDING PRESSURES:
ZONE 1: +13.0 / -37.0
ZONE 2: +13.0/ -43.9
ZONE 3: +16.5 / -102.2
ZONE 4: +30.2 / -33.6
ZONE 5: +36.2 / -45.7

DESIGN PARAMETERS E

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION.

IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, IN INITIAL CHEMICAL SOIL TREATMENT INSIDE THE FOUNDATION PERIMETER SHALL BE DONE AFTER ALL EXCAVATION, BACKFILLING AND COMPACTION IS COMPLETE.

SOIL AREA DISTURBED AFTER INITIAL CHEMICAL SOIL TREATMENT SHALL BE RETREATED WITH CHEMICAL SOIL TREATMENT, INCLUDING SPACE BOXES OR FORMED.

SPACE IN CONCRETE FLOORS BOXED OUT OR FORMED FOR THE SUBSEQUENT INSTALLATION OF PLUMBING TRAPS, DRAINS OR ANY OTHER PURPOSE SHALL BE CREATED BY USING PLASTIC OR MENTAL PERMANENTLY PLACED FORMS OF SUFFICIENT DEPTH TO ELIMINATE ANY PLANNED SOIL DISTURBANCE AFTER INITIAL CHEMICAL SOIL TREATMENT.

CHEMICALLY TREATED SOIL SHALL BE PROTECTED WITH A MINIMUM 6 MIL VAPOR RETARDER TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED. ANY WORK, INCLUDING PLACEMENT OF REINFORCING STEEL, DONE AFTER CHEMICAL TREATMENT UNITE THE CONCRETE FLOOR IS POURED, SHALL BE DONE IN SUCH MANNER AS TO AVOID PENETRATING OR DISTURBING TREATED SOIL.

CONCRETE OVER POUR OR MORTAR ACCUMULATED ALONG THE EXTERIOR FOUNDATION PERIMETER SHALL BE REMOVED PRIOR TO EXTERIOR CHEMICAL SOLID TREATMENT, TO ENHANCE VERTICAL PENETRATION OF THE CHEMICAL.

CHEMICAL SOIL TREATMENTS SHALL ALSO BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1 FOOT (305 MM) OF THE PRIMARY STRUCTURE SIDEWALLS. ALSO, A VERTICAL CHEMICAL BARRIER SHALL BE APPLIED PROMPTLY AFTER CONSTRUCTION IS COMPLETED, INCLUDING INITIAL LANDSCAPE AND IRRIGATION / SPRINKLER INSTALLATION. ANY SOIL DISTURBED AFTER THE CHEMICAL VERTICAL BARRIER IS APPLIED SHALL BE PROMPTLY RETREATED.

ALL BUILDINGS SHALL HAVE PRE-CONSTRUCTION TREATMENT PROTECTION AGAINST SUBTERRANEAN TERMITES. THE RULES AND LAWS AS ESTABLISHED BY FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES SHALL BE DEEMED AS APPROVED WITH RESPECT TO PRE-CONSTRUCTION SOIL TREATMENT FOR PROTECTION AGAINST SUBTERRANEAN TERMITES. A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT:

THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES.

PROTECTIVE SLEEVES AROUND METALLIC PIPING PENETRATING CONCRETE SLAB-ON-GRADE FLOORS SHALL NOT BE OF CELLULOSE-CONTAINING MATERIALS AND SHALL RECEIVE APPLICATION OF A TERMITICIDE IN ANNULAR SPACE BETWEEN SLEEVE AND PIPE.

TERMITE PROTECTION D

DIMENSIONS NOTED ARE TO FACE OF CMU. REFER TO SHEET A1.0 FOR OTHER DIMENSIONS NOT OTHERWISE NOTED.
DRAWINGS SHALL NOT BE SCALED. ALL DIMENSIONS AND FIT SHALL BE DETERMINED AND VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCEMENT OF WORK.
DETAILS NOT FULLY OR SPECIFICALLY SHOWN SHALL BE OF SAME NATURE AS OTHER SIMILAR CONDITIONS.
SEE PLUMB. DWGS. FOR PLUMB. LAYOUT DIMENSIONS, U.O.N.
SEE ELECT. DWGS. FOR ELECT. LAYOUT DIMENSIONS, U.O.N.
COORD. FOUNDATION AND SLAB LAYOUT WITH OTHER TRADES PRIOR TO POURING SLAB.

ALL SOILS WORK SHALL BE DONE IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS MANUAL. ALL FOUNDATIONS SHALL BEAR ON FIRM, UNDISTURBED, NATIVE SOILS OR ENGINEERED FILL AT OR EXCEEDING DEPTHS SHOWN ON THE DRAWINGS. INCREASE DEPTH AS REQUIRED BY GEOTECHNICAL ENGINEER. ALL FOOTING EXCAVATIONS SHALL BE AS NEAT AS PRACTICAL. OVER-EXCAVATIONS IN DEPTH SHALL BE FILLED WITH CONCRETE. OVER-EXCAVATIONS IN WIDTH MAY BE FILLED WITH LEAN CONCRETE OR APPROVED COMPACTED BACKFILL. ALL LOOSE SOILS SHALL BE REMOVED FROM EXCAVATIONS PRIOR TO PLACEMENT OF REINFORCING OR CONCRETE. ASSUMED ALLOWABLE DESIGN BEARING PRESSURE IS;
2000 PSF TO BE CONFIRMED WITH A LICENSED GEOTECHNICAL ENGINEER.

CONCRETE:
A. CONCRETE SHALL BE AS SPECIFIED (SEE SECTION 03300) AND MEET THE FOLLOWING MINIMUM ULTIMATE COMPRESSIVE STRENGTHS AT 28 DAYS:

LOCATION	MIN. STRENGTH 28 DAYS PSI	AGGREGATE SIZE - INCHESS	SLUMP INCHES	TOLERANCE
SLAB ON GRADE	(4000 DESIGN)	1" x #4	5"	±1/2"
FOUNDATIONS	(4000 DESIGN)	1" x #4	5"	±1/2"

B. CONCRETE MIX DESIGN AND TESTING SHALL MEET WITH THESE SPECS. CEMENT SHALL BE IN ACCORDANCE WITH ASTM C 150 TYPE I or TYPE III. VERIFY MINIMUM CONCRETE STRENGTH AND CEMENT TYPE.
C. REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60. STEEL SHALL BE KEPT CLEAN AND FREE OF RUST.
D. CONCRETE CURING SHALL BE IN ACCORDANCE WITH REQUIREMENTS OF ACI 318-11 SECTION 5.11 AND STANDARD PRACTICE FOR CURING CONCRETE REPORTED BY COMMITTEE 308.

SLAB:
A. 4" THICK CONCRETE SLAB WITH WWF REINFORCING OVER 15 MIL. POLYETHYLENE MEMBRANE OVER ENGINEERED SUBGRADE PER SOILS REPORT (TYP).
NOTE: FIBERMESH REINFORCING MAY BE SUBSTITUTED FOR WWF.

FOUNDATION PLAN NOTES C

1. SLAB SHALL BE SLOPED 1/2" TO DRAIN AT 4'-0" x 4'-0" SLAB HOLD-OUTS. TYPICAL AT ALL FLOOR DRAINS, U.O.N. AT MOP SINK LOCATIONS; SEE KEYNOTE 8, THIS SHEET. COORDINATE WITH PLUMBING DRAWINGS FOR ROUGH-IN LOCATIONS.
2. SHEAR WALL HOLDOWN; REFER TO SHEAR WALL SCHEDULE ON S3.0 AND DETAILS ON S5.0.
3. METAL STUDS @ KITCHEN HOOD, ATTACH BASE PER 15/S5.0.
4. ANCHOR BOLTS LOCATED THROUGHOUT PERIMETER OF BUILDING SHALL BE PROVIDED AS REQUIRED PER THE "PLATE/ANCHOR BOLT" COLUMN OF THE "WALL SHEATHING AND SHEARWALL SCHEDULE". SEE D/S3.0.
5. SEE 14/S4.0 FOR REINFORCING AT CORNERS AND FOOTING INTERSECTIONS
6. PROVIDE FOR HUB DRAIN (HD) ROUGH-IN; UNLESS LOCAL CODE REQUIRES FLOOR SINK (FS). COORDINATE LOCATION / SPECIFICATION WITH PLUMBING DRAWINGS.
7. INDICATES INSIDE SURFACE OF FOOTING. SEE SHEET S5.0.
8. PROVIDE FOR FLOOR DRAIN, FLOOR SINK, TRENCH DRAIN, OR FLOOR CLEAN-OUT ROUGH-IN; COORDINATE LOCATION / SPECIFICATION WITH PLUMBING DRAWINGS.
9. COOLER/FREEZER BY SUPPLIER

KEY NOTES N.T.S. B

LIS ARCHITECTURE ENGINEERING
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AA 26002040 CA ND: 6853 LB1057

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THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY ROBERT WAYNE CASE, PE ON JUNE 24, 2022. USING A DIGITAL SIGNATURE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

Digitally signed by ROBERT W CASE
Date: 2022.09.12 12:10:28 -04'00'

ROBERT WAYNE CASE
FLORIDA PE. #44643

PLAN SET REVISIONS:
1 06/22/22 - BUILDING COMMENTS
2 09/12/22 - BUILDING COMMENTS

CONTRACT DATE:	12-18-2019
BUILDING TYPE:	Kb 30-19
PLAN VERSION:	
SITE NUMBER:	
ENTITY NUMBER:	
STORE NUMBER:	
LIS PROJECT;	2019-303

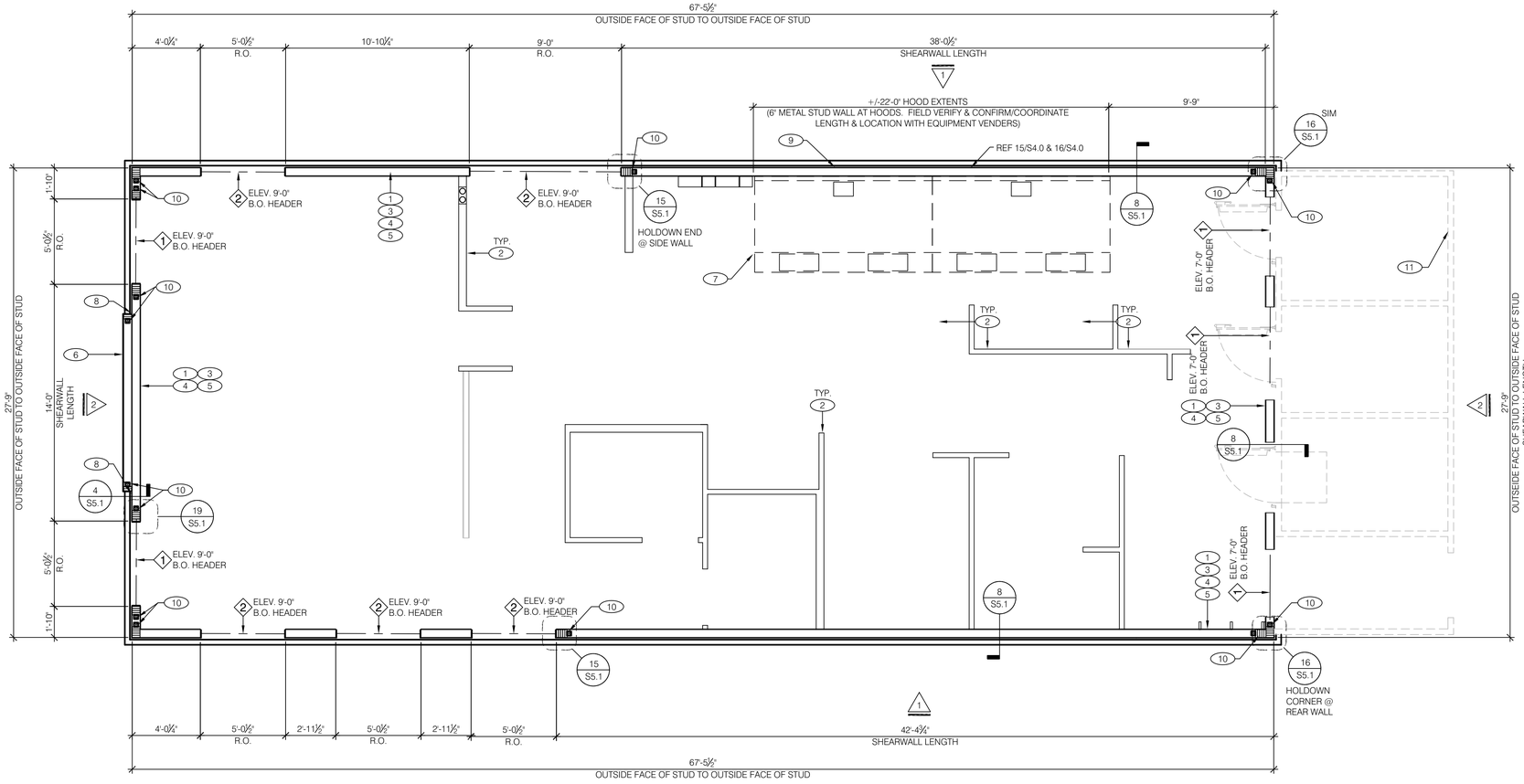
KFC
OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

Kb 30-19

FOUNDATION PLAN

S2.0

PLOT DATE: 03/29/2022



12 11 10 9 8 7 6 5 4 3 2 1

WALL FRAMING PLAN 1/4"=1'-0" A

MARK	BUILT-UP SECTION	MANUF. MEMBER
1	(3) 2 x 10 w/ 2x6 TOP AND BOTTOM	
2		(3) 1-3/4" x 11-1/4" LVL w/ 2x6 TOP AND BOTTOM

1. BUILT-UP HEADER SECTION SHALL HAVE ½" PLYWOOD SANDWICHED PIECES. SEE DETAIL 14, SHEET S4.2 FOR CONNECTION.
2. MANUFACTURED LUMBER SHALL 1.9E MICROLAM LVL OR APPROVED EQUAL (NER-200). SEE DETAIL 18, SHEET S4.2 FOR CONNECTION.
3. VERIFY ALL BOTTOM OF HEADER ELEVATIONS W/ ARCH. PRIOR TO CONSTRUCTION.

HEADER SCHEDULE AND NOTES F

SW	SHEATHING	EDGE	FIELD	HOLDOWN	PLATE / ANCHOR BOLT	REMARKS
◁	15/32" CDX PLYWD, PS1 RATING, TYP	10d @ 6" O.C.	10d @ 12" O.C.	HDU2 5/8" Ø ANCHOR DRILL AND EPOXY 6" EMBED W/ HILTI HY 200	5/8" Ø TITEN HD (7" EMBED) @ 32" O.C. W/ WASHER	PLYWOOD ON EXTERIOR FACE OF STUDS
▷	15/32" CDX PLYWD, PS1 RATING, TYP	10d @ 4" O.C.	10d @ 12" O.C.	HDU14 1" Ø ANCHOR 16" EMBED	5/8" Ø TITEN HD (7" EMBED) @ 16" O.C. W/ WASHER	PLYWOOD ON BOTH FACES OF STUDS
***	1/2" CDX PLYWD (32/16), PS1 RATING	10d @ 6" O.C.	10d @ 12" O.C.	N/A	5/8" Ø TITEN HD @32" OC W/ WASHER (7" EMBED)	NOT REQ'D

*** REQUIREMENTS FOR EXTERIOR NON-SHEARWALL WALLS

1. OSB OF COMPARABLE THICKNESS MAY BE USED IN LIEU OF PLYWOOD WHEN APPROVED IN WRITING BY THE PROJECT ENGINEER AND THE LOCAL JURISDICTION.
2. BLOCK ALL UNSUPPORTED EDGES WITH 2x MATERIAL U.O.N. BLOCK EDGES WITH 3x MATERIAL WHERE 10d NAILING IS 3" O.C. OR LESS AND 8d NAILING IS 2" O.C. OR LESS.
3. ALL PLYWOOD NAILS SHALL BE COMMON WIRE. SEE SPECIFICATIONS FOR OTHER NAIL REQUIREMENTS.
4. EXTERIOR WALLS NOT DESIGNATED AS SHEARWALLS IN THE WALL FRAMING PLAN SHALL MEET REQUIREMENTS INDICATED FOR NON-SHEARWALL WALLS IN THE SCHEDULE ABOVE.
5. SHEARWALL LENGTHS WHERE NOTED ARE MINIMUM. DO NOT LOCATE HOLDOWNS FROM THESE DIMENSIONS. SEE ARCH DWGS FOR ACTUAL WALL LENGTHS.
6. HD REFERS TO SIMPSON STRONGTIE CO. HOLDOWNS. INSTALL PER DETAILS ON S4.0. POST WIDTH SHALL MATCH STUD WALL WIDTH. SEE FOUNDATION PLAN FOR OTHER REQ'S.
7. EDGE NAIL WALL PLYWOOD TO STUDS OR POSTS WITH HOLDOWNS.
8. WHERE PANELS ARE APPLIED TO BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 4" O.C. ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO WALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3x OR THICKER AND NAILS ON EACH SIDE SHALL BE STAGGERED. SILL PLATES SHALL BE (2) 2x MEMBERS.
9. PROVIDE A MINIMUM OF TWO SILL ANCHORS PER SILL PLATE SEGMENT WITH BOLTS LOCATED FROM THE END OF EACH SEGMENT AT LEAST 4 INCHES, BUT NOT MORE THAN 12 INCHES.
10. FASTENERS FOR PRESSURE-PRESERVATIVE AND FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT-DIPPED ZINC-COATED GALVANIZED STEEL. THE COATING WEIGHTS SHALL BE IN ACCORDANCE WITH ASTM A153.
11. PROVIDE 1/4"x3"x3" PLATE WASHER BETWEEN HEAD OF SILL ANCHOR BOTLS AND WOOD SILL PLATE AT ALL INTERMEDIATE SILL ANCHORS.

WALL SHEATHING, SHEARWALL SCHEDULE AND NOTES D

STRUCTURAL WOOD FRAMING SHALL MEET OR EXCEED THE FOLLOWING:

USE	WOOD TYPE	GRADE	FB/PSI	FCP/PSI	E/PSI
2X8	SOUTHERN PINE	#2	925	1,350	1,400,000
2X10	SOUTHERN PINE	#2	800	1,300	1,400,000
2X12	SOUTHERN PINE	#2	750	1,250	1,400,000
STUDS (2X6)	SOUTHERN PINE	#2	1,000	1,400	1,400,000
6x6	SOUTHERN PINE	#1	1,350	825	1,500,000

ALL BEAMS AND JOISTS SHALL BE SEAT CUT FOR FULL UNIFORM BEARING AT SUPPORTS, BEAM SEATS AND COLUMN CAPS.
SEE SHEET A1.0 FOR DIMENSIONS.
EXTERIOR STUD WALLS ARE 2x6 AT 16" O.C. U.O.N.
ALL WOOD IN CONTACT WITH CONC., STEEL OR GRADE SHALL BE PRESSURE TREATED.
ALL BOLTED OR NAILED STRAP CONNECTIONS SHALL HAVE AN EQUAL NUMBER OF BOLTS OR NAILS EACH SIDE OF THE SPLICE JOINT. THE FIRST BOLT OR NAIL FROM EACH SIDE OF THE SPLICED OR TREATED MEMBER SHALL BE EQUAL DISTANCE FROM THE SPLICE. STRAPS USING 16d NAILS ON 2x MATERIAL SHALL BE INSTALLED ON THE 1-1/2" EDGE OF THE MEMBER.
THE CONTRACTOR SHALL VERIFY THAT THE MOISTURE CONTENT OF ALL FRAMING LUMBER AND PLYWOOD MEET THE REQUIREMENTS OF THE SPECS. AT THE TIME OF INSTALLATION AND AT CLOSE-IN.
USE AT PRESSURE TREATED LUMBER AT WINDOW JAMBS, SILL AND STUDS UNDER SILL AS WELL AS ALL TOILET PLUMBING WALLS.

LAYOUT STUDS ON SIDEWALLS (LONG WALLS) STARTING AT REAR OF BUILDING TOWARDS FRONT.
LAYOUT STUDS ON ENDWALLS (SHORT WALLS) STARTING AT EACH END AND WORKING TOWARDS CENTER.
LAYOUT STUDS TO ALIGN WITH ROOF TRUSSES. PROVIDE DOUBLE STUDS AT DOUBLE TRUSSES, TYPICAL. SEE ROOF FRAMING PLAN ON SHEET S3.0 FOR TRUSS LOCATIONS.

FRAMING PLAN NOTES C

1. SET OUTSIDE FACE OF STUD 1/2" IN FROM OUTSIDE FACE OF CONCRETE CURB.
2. INTERIOR NONBEARING STUD WALL FRAMING REFER TO SHEET A1.0 FOR DIMENSIONS, STUD SIZES, WALL ASSEMBLY. SEE DETAIL 13, 14/SS.1 AND WALL FRAMING NOTES.
3. (2) 2 x 6 TOP PLATES @ PARAPET 18'-5"; TYPICAL U.N.O. SPLICE PER 11/SS.1.
4. TOP OF PLATE ELEVATION 11'-0" = TRUSS BEARING.
5. (2) 2 x 6 BEARING W/ (3) FULL HEIGHT STUDS; TYPICAL AT WALL OPENING / WALL TERMINATION, U.N.O.
6. TOWER WALL FRAMING. OUTSIDE FACE OF EXTERIOR PLYWOOD SHEATHING SHALL BE SET FLUSH WITH EXTERIOR FACE OF THICKENED CONCRETE CURB.
7. HOOD, REFERENCE ARCH AND MEP. REFER TO 20/SS.1 FOR ATTACHMENT
8. EXTERIOR SHEATHING SHALL NOT BE INTERRUPTED WITH TOWER FRAMING OR INTERSECTING WALLS, TYPICAL.
9. FASTENER FOR SHEARWALL SHALL BE #8 SCREW AT SAME EDGE AND FIELD SPACING LISTED IN THE SHEARWALL SCHEDULE WHERE STUDS ARE METAL
10. SHEAR WALL HOLD DOWN, REFER TO KEY NOTE 2/S2.0. SEE SHEAR WALL SCHEDULE, THIS SHEET.
11. COOLER / FREEZER, BY SUPPLIER

KEY NOTES B

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ROBERT W. CASE LICENSE No. 44643 STATE OF FLORIDA PROFESSIONAL ENGINEER

Digitally signed by ROBERT W CASE
Date: 2022.09.12 12:10:49 -04'00'

ROBERT WAYNE CASE
FLORIDA PE. #44643

PLAN SET REVISIONS:

1. 06/22/22 - BUILDING COMMENTS
2. 09/12/22 - BUILDING COMMENTS

CONTRACT DATE:	12-18-2019
BUILDING TYPE:	Kb 30-19
PLAN VERSION:	
SITE NUMBER:	
ENTITY NUMBER:	
STORE NUMBER:	
LIS PROJECT;	2019-303

KFC
OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

Kb 30-19

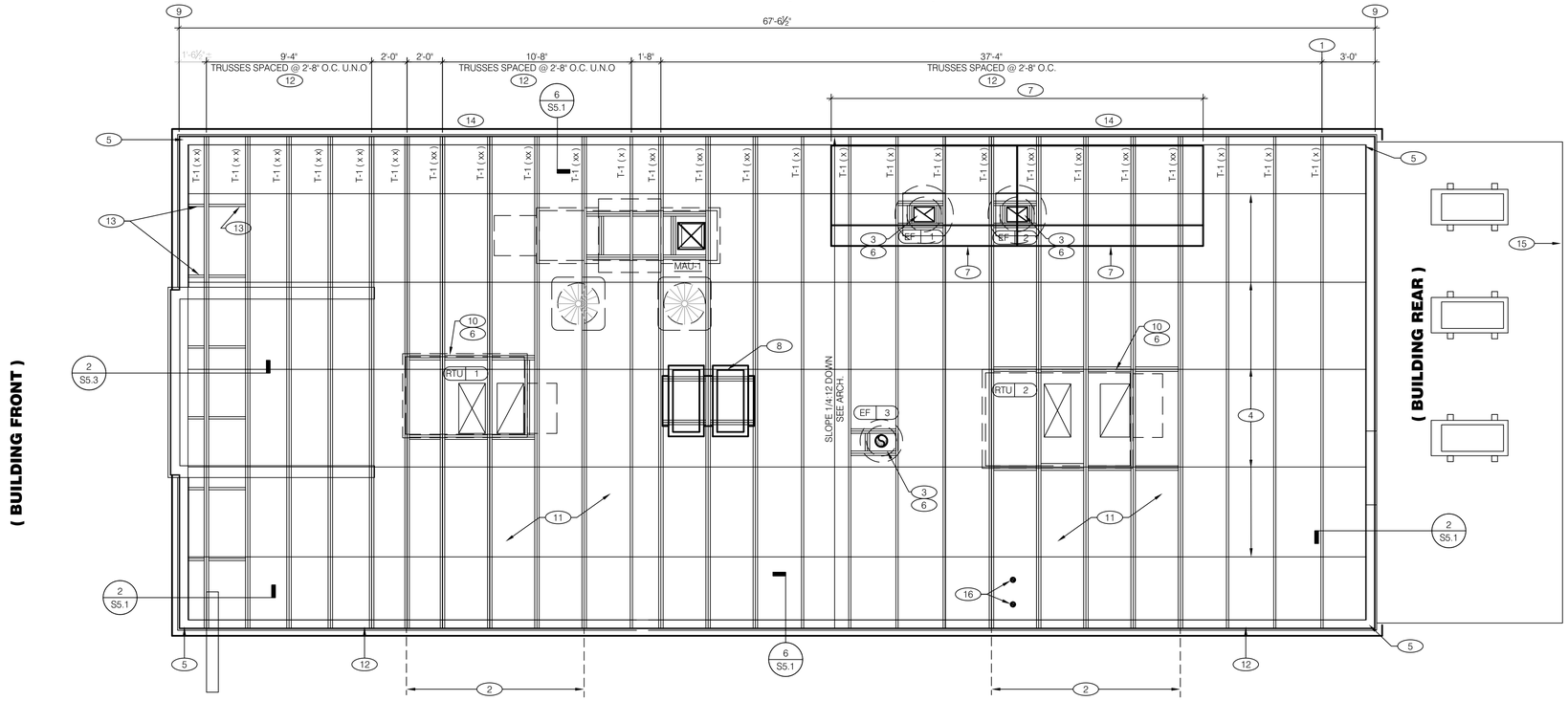
WALL FRAMING PLAN

S3.0

PLOT DATE: 03/29/2022

B-22-511575.000
REV: 9/12/2022 2:26 PM

(BUILDING LEFT)



(BUILDING RIGHT)

EXTREME CARE SHALL BE USED IN ERECTING ROOF TRUSSES - COMPLY WITH TPI BRACING REQUIREMENTS.

ROOF NOT DESIGNED FOR PONDING. SEE ARCHITECTURAL DRAWINGS FOR DRAIN REQUIREMENTS. VERIFY ROOF SLOPES W/ ARCHITECTURAL DRAWINGS.

12 11 10 9 8 7 6 5 4 3 2 1

ROOF FRAMING PLAN 1/4"=1'-0" A

TYPE	NAILING / SHEATHING	REMARKS
BN	10d @ 4" O.C.	-
EN	10d @ 6" O.C.	-
FN	10d @ 12" O.C.	-
ROOF SHEATHING	5/8" CDX PLYWOOD (40/20), PS1 RATING	

NOTE: SEE 11/S5.2 FOR DEFINITIONS.

NAILING SCHEDULE - ROOF D

ROOF FRAMING NOTES:

A. ALL UNSUPPORTED EDGES OF PLYWOOD SHEATHING SHALL BE BLOCKED WITH 2x4 INSTALLED FLAT. PLYWOOD METAL CLIPS ARE NOT PERMITTED. SEE DETAIL 8 / S5.2. OSB OF COMPARABLE THICKNESS MAY BE USED IN LIEU OF PLYWOOD WHEN APPROVED IN WRITING BY THE PROJECT ENGINEER AND THE LOCAL JURISDICTION.

B. ALL MECHANICAL SUPPLY AND RETURN OPENINGS SHALL BE BETWEEN FRAMING U.O.N.

MANUFACTURED ROOF TRUSS NOTES:

A. MANUFACTUTRED ROOF TRUSSES ARE AT 2'-8" O.C. U.O.N.

B. "T-# (x)" DENOTES ROOF TRUSS TYPE. REFER TO SCHEDULE 13 / S5.2.

C. TRUSS DRAWINGS ARE PROVIDED FOR CONCEPTUAL DESIGN ONLY. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS, BOTH SIGNED BY A LICENSED STRUCTURAL ENGINEER (STATE OF PROJECT). SUBMIT SHOP DRAWINGS AND CALCULATIONS TO THE ARCHITECT AND ENGINEER OF RECORD FOR REVIEW, AND IF REQUIRED, TO BUILDING OFFICIAL FOR APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS SHALL INCLUDE LAYOUT PLAN AND CONNECTORS. CALCULATIONS SHALL BE BASED ON THE SPECIFIED LOADING CONDITIONS SHOWN HEREIN. TRUSS MANUFACTURER SHALL PROVIDE HANGERS AND CONNECTIONS BETWEEN TRUSSES. REVIEW AND APPROVE DIMENSIONS, SHAPES AND DETAILS SHOWN ON SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT / ENGINEER FOR REVIEW AND COMMENT. ALLOW A MINIMUM OF 10 BUSINESS DAYS FOR REVIEW.

D. TRUSS MANUFACTURER SHALL PROVIDE HANGERS AND CONNECTORS ADEQUATE FOR LOADS. ROOF CONNECTORS ARE BASED UPON SIMPSON "STRONG TIE" OR APPROVED EQUAL.

E. TRUSS CHORDS AND PARAPET VERTICALS SHALL BE 2 x 6 MINIMUM AND CONSISTENTLY SIZED THROUGHOUT PROJECT.

F. REFER TO TRUSS ELEVATIONS FOR SHAPE, VARIATION, SPAN, ETC. LOCATION OF BEARING POINTS ARE AS INDICATED ON THE DRAWINGS.

G. MANUFACTURERED ROOF TRUSS DESIGN LOADS: SEE TRUSS DESIGN CRITERIA 9 / S5.2.

H. THE POSITIONS, WEIGHTS, AND METHODS OF ATTACHMENT OF ALL MECHANICAL UNITS, ELECTRICAL FIXTURES, PLUMBING, ETC. SHALL BE INCLUDED IN THE DESIGN OF THE TRUSSES BY THE TRUSS MANUFACTURER.

I. DESIGN ROOF TRUSSES TO SUPPORT ALL IMPOSED LOADS, INCLUDING WIND & LATERAL LOADS. COORDINATE SIZE, LOCATION AND WEIGHT OF EQUIPMENT WITH MECHANICAL WORK. PROVIDE MULTIPLE TRUSSES WHERE ONE TRUSS CANNOT SUPPORT THE LOAD. PROVIDE BRIDGING BETWEEN TRUSSES AS SPECIFIED AS MINIMUM STANDARD.

J. INSTALLATION OF ALL TRUSSES SHALL BE DONE USING A SPREADER BAR WITH A THREE POINT VERTICAL PICK. CARE SHALL BE USED IN LIFTING TO PREVENT HORIZONTAL BENDING.

K. IMPROPER HANDLING OF THE TRUSSES AS NOTED ABOVE AND IN THE SPECS SHALL MEAN REMOVAL OF THE TRUSSES FROM THE JOBSITE AND REPLACEMENT AT CONTRACTOR'S EXPENSE.

L. SEE DIV. 6 OF THE SPECS FOR DETAILS ON TRUSS MANUFACTURING AND NAILING.

ROOF FRAMING NOTES C

1. STARTING POINT OF TRUSS LAYOUT - CENTERLINE OF TRUSS / DOUBLE TRUSS.
2. VERIFY NECESSITY OF DOUBLE TRUSSES WITH TRUSS MFR. DUE TO POINT LOADING, TYPICAL.
3. FRAMING FOR EXHAUST FAN CURB. VERIFY SIZE OF OPENING / FRAMING LOCATION WITH EXHUAST FAN MANUFACTURER and MECHANICAL DRAWINGS.
4. CONTINUOUS 2 x 4 WOOD BRIDGING ON TOP OF BOTTOM CHORD. ADJUST AS REQUIRED FOR DUCT PLENUMS, MAXIMUM SPACING AT 5'-0" O.C. OR TIGHTER SPACING AS REQUIRED BY TRUSS DESIGN. SEE 12 / S5.1 FOR LAP CONFIGURATION.
5. SIMPSON MSTA 24 AT CORNER DBL TOP PLATE. CENTER STRAP ON CORNER.
6. (2) 2x6 JOISTS W/ U26-2 HANGERS; EACH END. TYPICAL AT ALL ROOF OPENINGS. SEE DETAIL 17 / S5.2.
7. LOCATION OF HOODS. SEE HOOD DRAWINGS FOR HOOD ATTACHMENT DETAILS.
8. FRAMING FOR CONDENSER PLATFORM. REFER TO SHEET A3.0.
9. DIMENSIONS ARE FROM OUTSIDE FACE OF EXTERIOR WALL FRAMING.
10. FRAMING FOR RTU CURB. VERIFY SIZE OF OPENING / FRAMING LOCATION WITH RTU MANUFACTURER AND MECHANICAL DRAWINGS.
11. ROOF SHEATHING. REFER TO D/S4.0.
12. TRUSS BEARING (B.O. TRUSS) @ 11'-0" A.F.F. TYPICAL.
13. 2 x 6 DIAGONAL BRACING @ 4'-0" O.C. SEE DETAIL 1 / S5.1.
14. FRAMING FOR ROOF DRAIN; COORDINATE WITH ROOF DRAIN MANUFACTURER SPECIFICATIONS / REQUIREMENTS.
15. OUTLINE OF COOLER/FREEZER ROOF
16. WATER HEATER EXHAUST AND INTAKE FLUE

KEY NOTES B

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Digitally signed by ROBERT W CASE
Date: 2022.09.12 12:11:13 -04'00'

ROBERT WAYNE CASE
FLORIDA PE. #44643

PLAN SET REVISIONS:

1. 06/22/22 - BUILDING COMMENTS
2. 09/12/22 - BUILDING COMMENTS

CONTRACT DATE:	12-18-2019
BUILDING TYPE:	Kb 30-19
PLAN VERSION:	
SITE NUMBER:	
ENTITY NUMBER:	
STORE NUMBER:	
LIS PROJECT;	2019-303

KFC

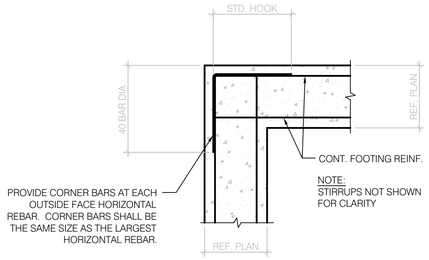
OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

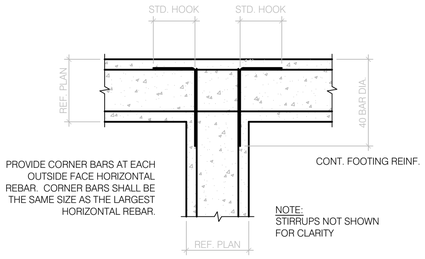
Kb 30-19

ROOF FRAMING PLAN

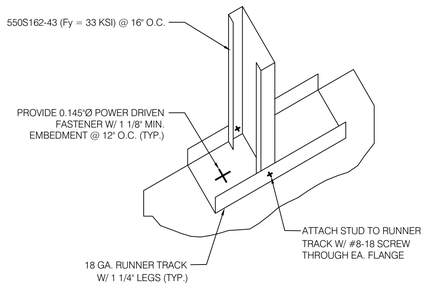
S4.0

PLOT DATE: 03/29/2022

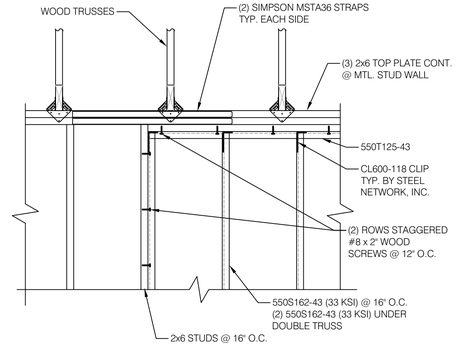




CORNER & INTERSECTING REINF. N.T.S. **14**

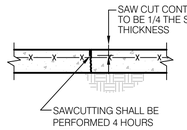


METAL STUD WALL BASE N.T.S. **15**

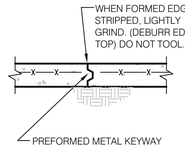


METAL STUD FRAMING N.T.S. **16**

NOTES:
1. FILL JOINT IF CRACK IS OVER 1/8" WIDE.
2. JOINT SPACING SHALL NOT EXCEED 12'-0" O.C.
3. CONSTRUCTION JOINTS, IF REQUIRED BY THE CONTRACTOR, SHALL BE LOCATED AT A CONTROL JOINT LOCATION.

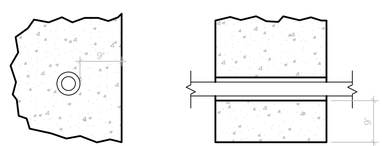


TYPICAL CONTROL JOINT



TYPICAL CONSTRUCTION JOINT

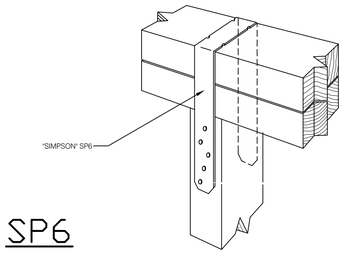
TYP. CONST. & CONTROL JOINT DETAILS N.T.S. **9**



ELEVATION SECTION

NOTES
1. SLEEVES SHALL BE MINIMUM 1" CLEAR ALL AROUND PIPES, CONDUIT, ETC.
2. COORD REQUIREMENTS OF SLAB VENTING WITH SOILS ENGINEER, IF REQUIRED.

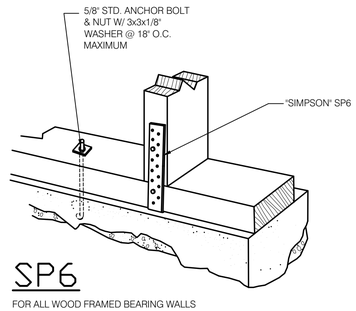
PIIPES THROUGH FOUNDATIONS N.T.S. **10**



SP6

FOR USE AT ALL WOOD FRAMED EXTERIOR WALLS

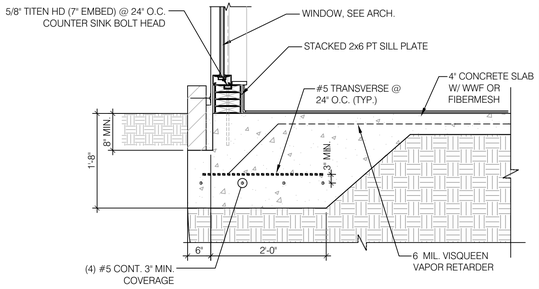
TOP PLATE CONNECTOR N.T.S. **11**



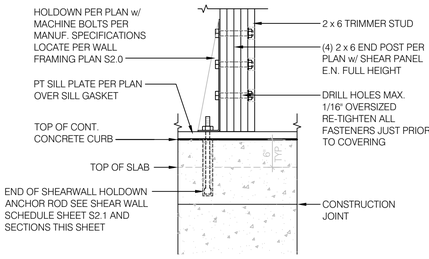
SP6

FOR ALL WOOD FRAMED BEARING WALLS

BOTTOM PLATE CONNECTOR N.T.S. **12**

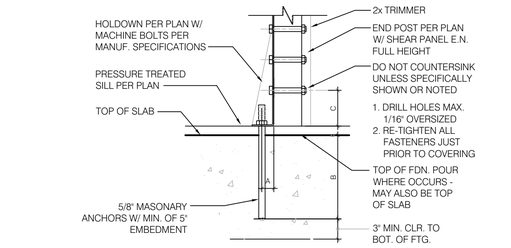


NEW FOOTING @ LOW WINDOW 3/4"=1'-0" **5**



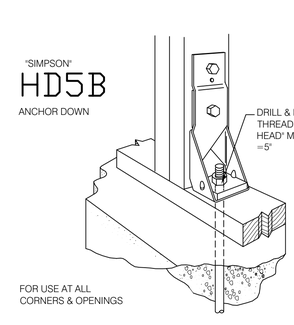
NOTE: REFER TO SHEAR WALL SCHEDULE, S2.0.

SHEARWALL HOLDDOWN 3/4" = 1'-0" **6**



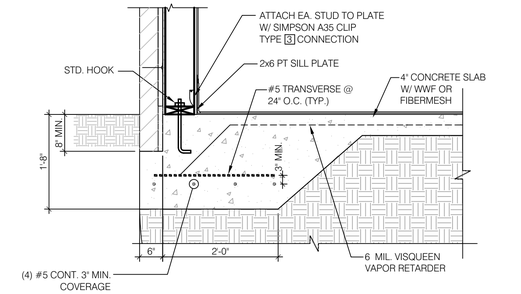
* INCREASE FOOTING DEPTH AS REQ'D FOR ANCHOR BOLT EMBEDMENT
** ANCHOR BOLT LENGTH TO EXTEND MIN. 5" BEYOND CONST. JOINT

ANCHOR BOLT SCHEDULE N.T.S. **7**

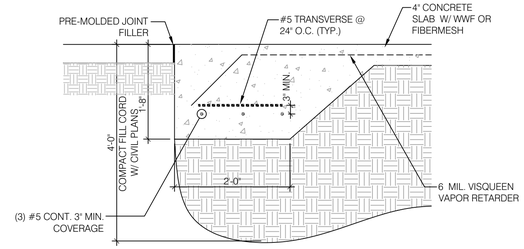


FOR USE AT ALL CORNERS & OPENINGS

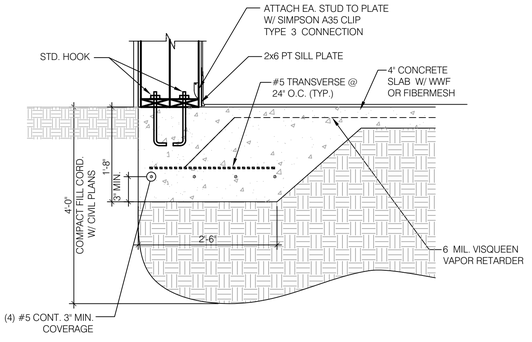
ANCHOR DOWN N.T.S. **8**



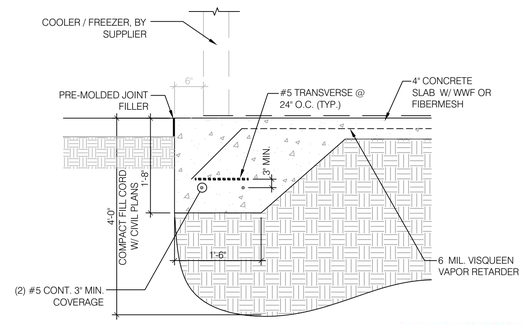
NEW FOOTING 3/4" = 1'-0" **1**



NEW FOOTING @ DOOR 3/4" = 1'-0" **2**



NEW FOOTING @ FRONT WALL SIGN 3/4" = 1'-0" **3**



FOOTING @ COOLER / FREEZER 3/4" = 1'-0" **4**

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KFC

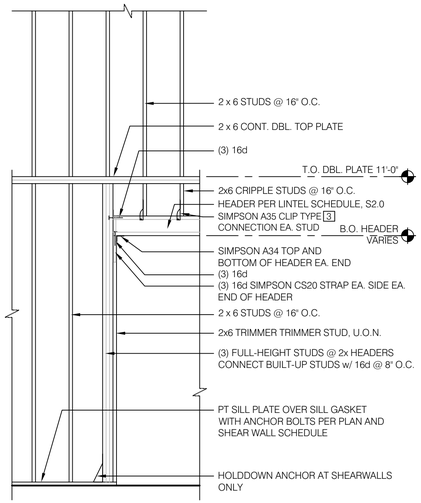
OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

Kb 30-19

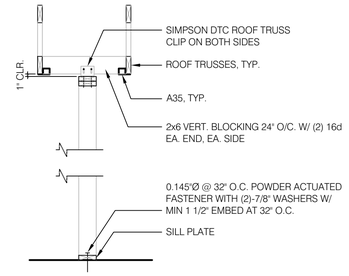
STRUCTURAL DETAILS (FOUNDATION)

S5.0

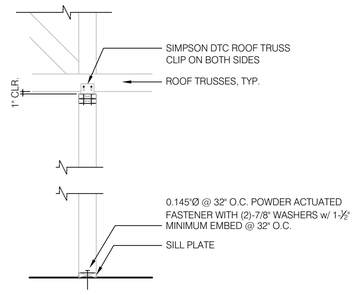
PLOT DATE: 03/29/2022



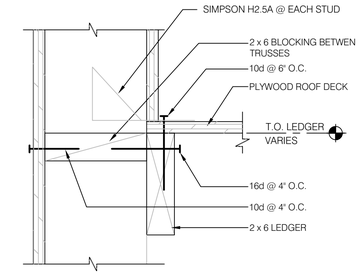
FRAMING ELEVATION @ OPENING 3/8"=1'-0" | **18**



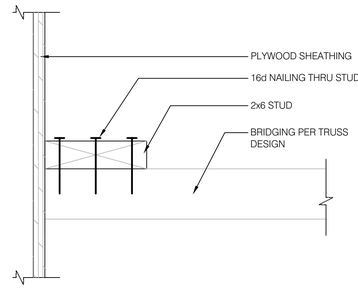
SUPPORT- PARALLEL TO TRUSS 1/2" = 1'-0" | **13**



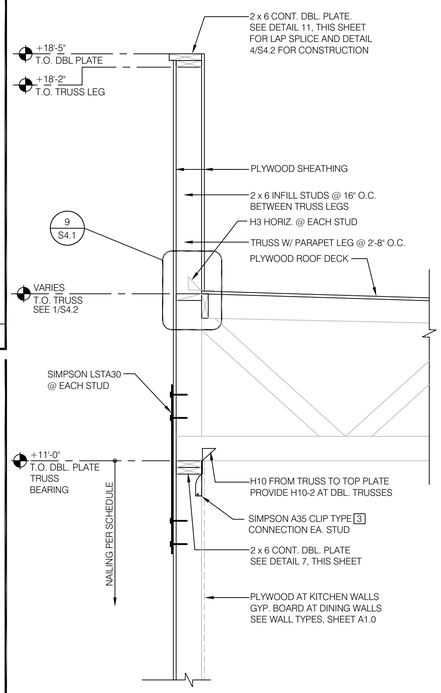
SUPPORT PERP. TO TRUSS 1/2" = 1'-0" | **14**



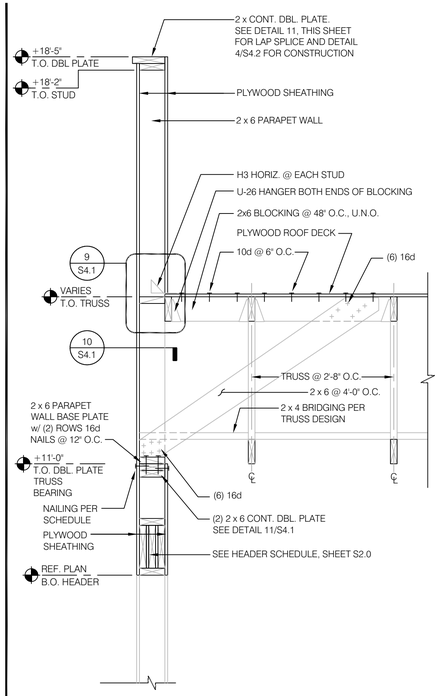
LEDGER DETAIL 3" = 1'-0" | **9**



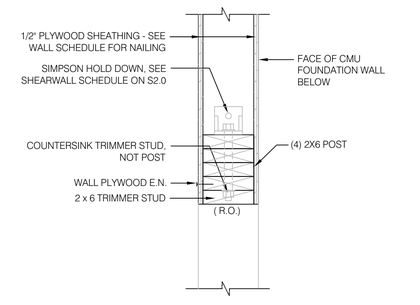
BRIDGING DETAIL 3" = 1'-0" | **10**



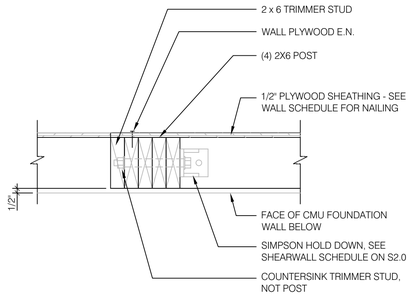
SIDE WALLS @ TRUSS 3/4" = 1'-0" | **6**



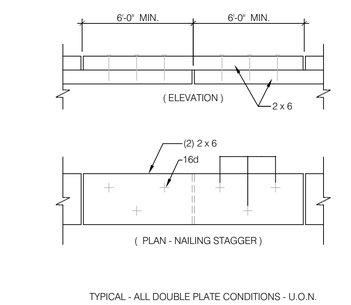
FRONT + REAR WALLS @ TRUSS 3/4" = 1'-0" | **2**



SW HOLD DOWN: END (FRONT) 1-1/2" = 1'-0" | **19**

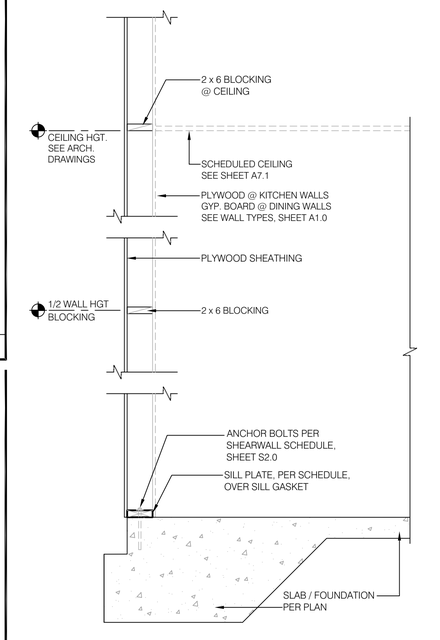


SW HOLD DOWN: END (SIDE) 1-1/2" = 1'-0" | **15**

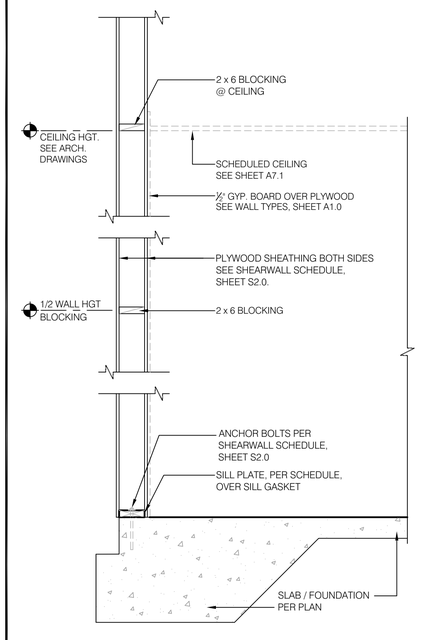


TYPICAL - ALL DOUBLE PLATE CONDITIONS - U.O.N.

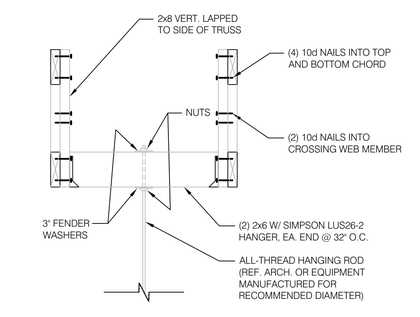
PLATE LAP DETAIL N.T.S. | **11**



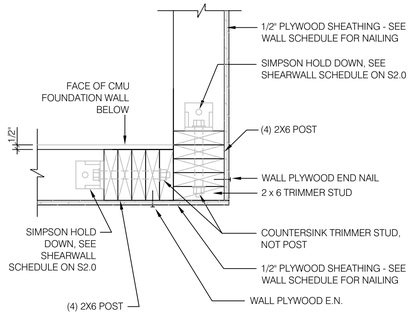
SIDE + REAR WALLS FRAMING 3/4" = 1'-0" | **8**



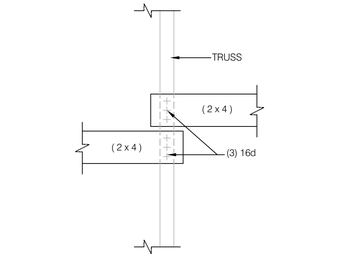
FRONT WALL FRAMING 3/4" = 1'-0" | **4**



HANGING BULKHD. OR HOOD DETL. N.T.S. | **20**



SW HOLD DOWN: CORNER (REAR) 1-1/2" = 1'-0" | **16**



NOTE: REFER TO TRUSS ELEVATION SHEET S4.2

BRIDGING LAP DETAIL N.T.S. | **12**

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Digitally signed by ROBERT W CASE
Date: 2022.09.12 12:11:55 -04'00'
ROBERT WAYNE CASE
FLORIDA PE. #44643

PLAN SET REVISIONS:

1. 06/22/22 - BUILDING COMMENTS
2. 09/12/22 - BUILDING COMMENTS

CONTRACT DATE:	12-18-2019
BUILDING TYPE:	Kb 30-19
PLAN VERSION:	
SITE NUMBER:	
ENTITY NUMBER:	
STORE NUMBER:	
LIS PROJECT;	2019-303

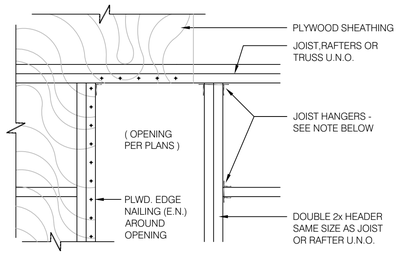
KFC
OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

Kb 30-19

STRUCTURAL DETAILS (FRAMING)

S5.1

PLOT DATE: 03/29/2022



NOTE:
HANGERS SHALL BE "SIMPSON" STANDARD "U" @ ROOF. DBL. JOIST OR RAFTERS SHALL BE JOINED TOGETHER AS FOLLOWS:
1. 10" OR LESS NOM. DEPTH - 16d @ 12" O.C. STAGGERED
2. MORE THAN 10" - (2)16d @ 12" O.C.

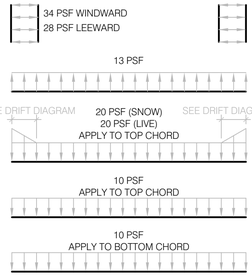
TYPICAL ROOF OPENING 1"=1'-0" | 17

TRUSS TYPES	SINGLE TRUSS DESIGNATION	DOUBLE TRUSS DESIGNATION	BEARING POINT	COMMENTS
T-1	X	XX	△	SEE NOTE 1.

NOTES:

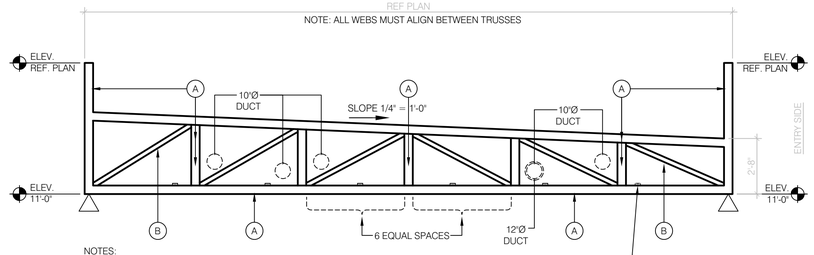
1. REFER TO TRUSS-TYPE ELEVATION 1, THIS SHEET. PROVIDE DOUBLE THICKNESS TRUSS (WHERE INDICATED "XX") AS REQUIRED BY TRUSS DESIGN. COORDINATE TRUSS-TYPES WITH SHEET S3.0.
2. HOLDDOWN CONNECTORS SHALL BE SPECIFIED BY SITE SPECIFIC ARCHITECT/ ENGINEER BASED UPON LOADING DATA PROVIDED BY TRUSS DESIGNER.
3. PROVIDE SHOP DRAWINGS PRIOR TO FABRICATION.
4. TRUSS MEMBER SIZES ARE FOR REFERENCE ONLY. ACTUAL SIZE SHALL BE DETERMINED BY TRUSS MANUFACTURER BASED ON ACTUAL LOAD CONDITIONS AND CODES.

TRUSS SCHEDULE 13



NOTE:
ALSO, APPLY ROOF TOP AND SUSPENDED POINT LOADS. WEIGHT AND LOCATION OF UNITS AS NOTED ARE SHOWN ON THIS SHEET AND ARE NOT INCLUDED IN THE ABOVE LOADING DIAGRAM. VERIFY THESE LOADS WITH MECHANICAL SUPPLIER BEFORE DESIGNING TRUSS.

TRUSS DESIGN CRITERIA N.T.S. | 9



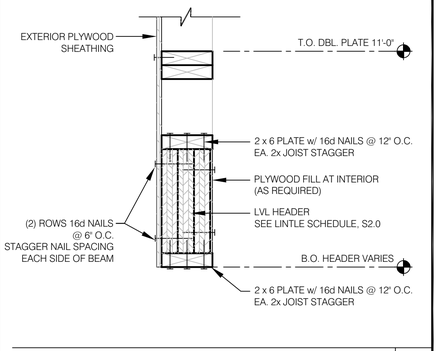
NOTES:

1. TRUSS DEPTH TO BE DETERMINED BY SITE-ADAPT ENGINEER OF RECORD.
2. REFER TO SHEET M2.0 FOR COORDINATION OF SUPPLY AIR DUCTS WITHIN FRONT OF HOUSE TRUSS WEB SPACE.

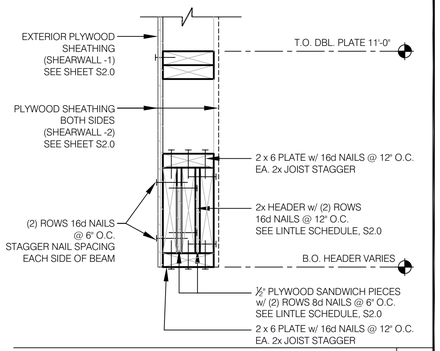
CONT 2X4 LATERAL BRACING AT 5'-0"O.C. MIN. OR PER TRUSS DESIGN LAID FLAT & NAILED TO TOP OF BOTTOM CHORD. APPLY MOST STRINGENT STANDARD.

A	2 x 6 MIN.
B	2 x 4 MIN.

TRUSS ELEVATION 1/4" = 1'-0" | 1



TYP. LVL HEADER 1 1/2" = 1'-0" | 18

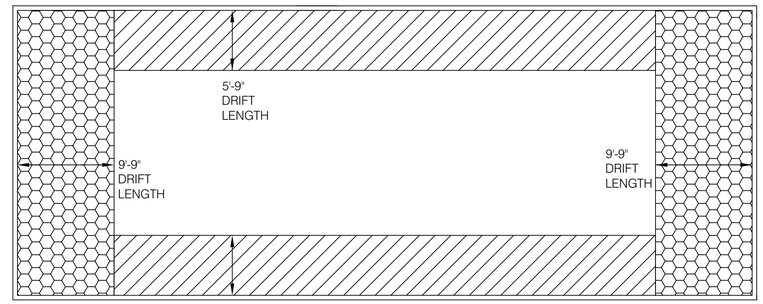


TYP. BUILT-UP HEADER 1 1/2"=1'-0" | 14

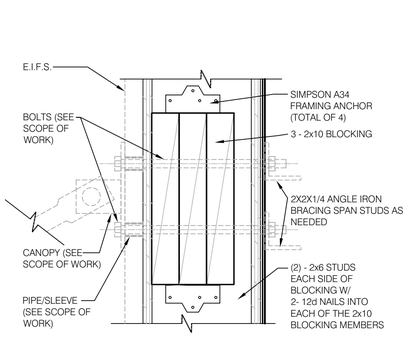
EQUIPMENT	DESIGN WEIGHT	NOTES
HVAC UNIT - RTU-1	2,600 lbs.	A, B
HVAC UNIT - RTU-2	2,600 lbs.	A, B
EXHAUST FAN - EF-1	200 lbs.	A
EXHAUST FAN - EF-2	200 lbs.	A
EXHAUST FAN - EF-3	100 lbs.	A
EXHAUST FAN - EF-4	50 lbs.	A
EXHAUST FAN - EF-5	50 lbs.	A
HOOD #1	690 lbs.	
HOOD #2	690 lbs.	
ICE CONDENSER #1	175 lbs.	
ICE CONDENSER #2	175 lbs.	
COOLER CONDENSER	175 ea	
MUZAK DISH, SLED & BALLAST - ALLOW FOR LOCATION ANYWHERE ON ROOF.	1,090 lbs.	

NOTE:
A. ALL DESIGN WEIGHTS INCLUDE CURB.
B. COORDINATE WEIGHTS WITH HVAC UNIT SCHEDULE, SEE 1 / M1.0,

ROOF TOP EQUIPMENT WEIGHTS 10



SNOW DRIFT LOADING N.T.S. | 3



CANOPY / TIE-ROD BLOCKING 3" = 1'-0" | 19

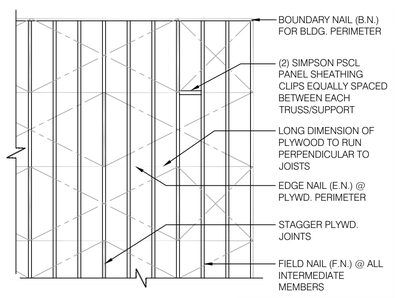
Design Wind Pressure (psf):			Effective Wind Area (sqft)					
Walls:			10	20	40	100	200	500
Interior	Area 4	+	17.9	17.1	16.3	16.0	16.0	16.0
		-	-19.4	-18.6	-17.8	-16.7	-16.0	-16.0
Edge	Area 5	+	17.9	17.1	16.3	16.0	16.0	16.0
		-	-23.8	-22.2	-20.7	-18.6	-17.0	-16.0
Roof:			10	20	50	100	200	500
Interior	Area 1	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-19.5	-19.0	-18.4	-17.9	-17.9	-17.9
Edge	Area 2	+	17.9	17.1	16.3	16.0	16.0	16.0
		-	-32.7	-29.3	-24.7	-21.2	-21.2	-21.2
Corner	Area 3	+	17.9	17.1	16.3	16.0	16.0	16.0
		-	-32.7	-29.3	-24.7	-21.2	-21.2	-21.2
Overhang:			10	20	50	100	200	500
Interior	Area 1	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-28.1	-27.6	-27.0	-26.5	-23.0	-18.3
Edge	Area 2	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-28.1	-27.6	-27.0	-26.5	-23.0	-18.3
Corner	Area 3	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-46.3	-36.4	-23.2	-16.0	-16.0	-16.0
Parapet Design Pressure (psf):			Effective Wind Area (sqft)					
Parapet:			10	20	50	100	200	500
Edge	Area 2	+	56.6	51.2	44.0	38.6	37.6	36.3
		-	-39.6	-37.6	-35.0	-33.0	-31.0	-28.3
Corner	Area 3	+	56.6	51.2	44.0	38.6	37.6	36.3
		-	-45.3	-42.3	-38.3	-35.3	-32.3	-28.3

WIDTH OF PRESSURE COEFFICIENT, a = 3'-9"

COMP. & CLADDING WIND PRESSURES 20

CONNECTION TYPE:	NAILING:
1. JOIST TO SILL OR GIRDER, TOENAIL	(3-8d)
2. BRIDGING TO JOIST, TOENAIL EACH END	(2-8d)
3. 1"x6" (25MMx152MM) SUBFLOOR OR LESS TO JOIST, FACE NAIL	(2-8d)
4. WIDER THAN 1" X 6"(25MMx152MM) SUBFLOOR TO JOIST, FACE NAIL	(3-8d)
5. 2" (52MM) SUBFLOOR TO GIRDER, BLIND AND FACE NAIL	(2-16d)
6. SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL	(16d @16" O.C.)
7. SOLE PLATE TO JOIST OR BLOCKING, AT BRACED W. PANELS	(3-16d PER 16")
8. TOP PLATE TO STUD, END NAIL	(2-16d)
9. STUD TO SOLE PLATE	(2-16d END NAIL)
10. DOUBLE STUDS, FACE NAIL	(16d @ 24", O.C.)
11. DOUBLE TOP PLATES, TYPICAL FACE NAIL	(16d @ 16" O.C.)
12. DOUBLE TOP PLATES, LAP SPLICE	(8-16d)
13. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL	(3-8d)
14. RIM JOIST TO TOP PLATE, TOENAIL	(8d @ 6" O.C.)
15. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	(2-16D)
16. CONTINUOUS HEADER, TWO PIECES	(16d @ 16" O.C. ALONG EDGE)
17. CEILING JOISTS TO PLATE, TOENAIL	(3-8d)
18. CONTINUOUS HEADER TO STUD, TOENAIL	(4-8d)
19. CEILING JOISTS, LAP OVER PARTITIONS, FACE NAIL	(3-16d)
20. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	(3-16d)
21. RAFTER TO PLATE, TOENAIL	(3-8d)
22. 1" (25MM) BRACE TO EACH STUD AND PLATE, FACE NAIL	(2-8d)
23. 1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	(2-8d)
24. WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL	(3-10d)
25. BUILT-UP CORNER STUDS	(16d @ 24" O.C.)
26. 2" PLANKS	(2-16d AT EACH SPLICE)
27. 2x6 BOX BEAM / HEADER	(12d @ 12" O.C.)
28. BUILT-UP GIRDER AND BEAMS	(20d @ 32" O.C. AT TOP & BOTTOM AND STAGGERED 2-20d AT ENDS AND AT EACH SPLICE)

NAILING SCHEDULE 16

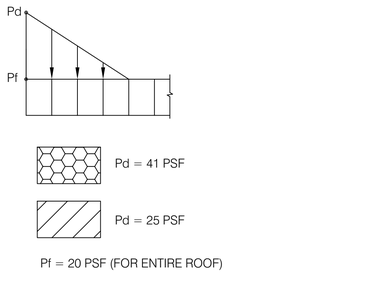


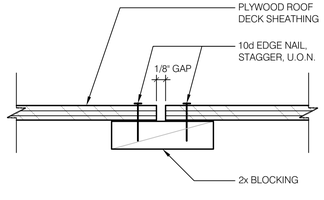
NOTES:
1. MIN. PLYWD. SHT. SIZE SHALL BE 2'-0" X 4'-0".
2. MIN. 3/8" NAILING EDGE DISTANCE.
3. EDGE NAIL (E.N.) O/ BEAMS AND AROUND ALL OPENINGS.

DEFINITIONS:
BN DENOTES BOUNDARY NAILING
EN DENOTES EDGE NAILING
FN DENOTES FIELD NAILING

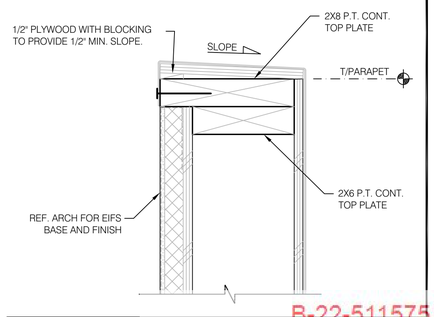
ROOF NAILING PLAN N.T.S. | 11

NOT USED 12





PLYWOOD EDGE BLOCKING N.T.S. | 8



TYP. PARAPET CAP 3" = 1'-0" | 4

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ROBERT W. CASE LICENSE No. 44643 STATE OF FLORIDA PROFESSIONAL ENGINEER

Digitally signed by ROBERT W CASE
Date: 2022.09.12 12:12:19 -04'00'

ROBERT WAYNE CASE
FLORIDA PE. #44643

PLAN SET REVISIONS:

1. 06/22/22 - BUILDING COMMENTS
2. 09/12/22 - BUILDING COMMENTS

CONTRACT DATE:	12-18-2019
BUILDING TYPE:	Kb 30-19
PLAN VERSION:	
SITE NUMBER:	
ENTITY NUMBER:	
STORE NUMBER:	
LIS PROJECT;	2019-303

KFC

OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

Kb 30-19

STRUCTURAL DETAILS (ROOF)

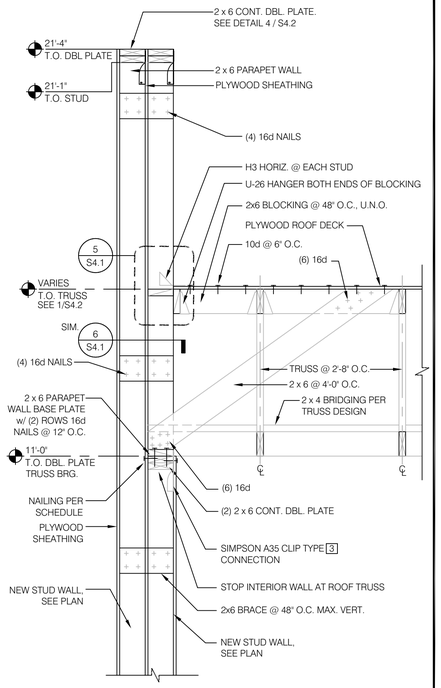
S5.2

PLOT DATE: 03/29/2022

B-22-511575.000
REV: 9/12/2022 2:26 PM

NOT USED NTS | 14

NOT USED NTS | 6



TOWER FRONT WALL @ TRUSS 3/4" = 1'-0" | 2

NOT USED NTS | 20

NOT USED NTS | 16

NOT USED NTS | 12

NOT USED NTS | 8

NOT USED NTS | 4

LIS ARCHITECTURE ENGINEERING

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Date: 2022.09.12 12:12:44 -04'00'

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KFC

OAKLEAF CORNER OUTPARCEL 3
DUVAL COUNTY, FL

Kb 30-19

STRUCTURAL DETAILS (FRAMING)

S5.3

PLOT DATE: 03/29/2022

B-22-511575.000
REV: 9/12/2022 2:26 PM