

GENERAL STRUCTURAL NOTES

APPLY UNLESS NOTED OTHERWISE ON DRAWINGS.

BUILDING CODE:

1985 EDITION OF THE UNIFORM BUILDING CODE.

LOADS:

ROOF LIVE LOAD = 20 PSF (REDUCIBLE).
 ROOF DEAD LOAD = 30 PSF.
 WIND LOAD = 70 MPH WIND SPEED ZONE, EXPOSURE C.
 SEISMIC ZONE 2 (Z = 0.3).
 SUPERIMPOSED DEAD LOAD ON ROOF TRUSSES = 27 PSF.

FOUNDATIONS:

SOIL REPORT BY WESTERN TECHNOLOGIES; JOB NO. 2128/1228. SPREAD FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED SOILS, RECOMPACTED NATIVE SOILS, AND/OR ENGINEERED FILL 1'-6" MINIMUM BELOW FINISHED GRADE. FINISHED GRADE IS DEFINED AS TOP OF SLAB FOR INTERIOR FOOTINGS AND LOWEST ADJACENT GRADE FOR PERIMETER FOOTINGS. DESIGN SOIL BEARING VALUE = 1,500 PSF. FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY SOILS ENGINEER PRIOR TO PLACEMENT OF CONCRETE.

CONCRETE

MINIMUM 28 DAY STRENGTH AS FOLLOWS:

SLABS ON GRADE	3,000 PSI
FOUNDATIONS	2,500 PSI

MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, EXCEPT THAT SLABS ON GRADE NEED BE VIBRATED ONLY AROUND UNDER-FLOOR DUCTS, ETC. MAXIMUM SLUMP 4 1/2". CAST CLOSURE POUR AROUND COLUMNS AFTER COLUMN DEAD LOAD IS APPLIED. UNLESS APPROVED OTHERWISE IN WRITING BY THE ARCHITECT, ALL CONCRETE SLABS ON GRADE SHALL BE BOUNDED BY CONTROL JOINTS (KEYED OR SAW CUT) SUCH THAT THE ENCLOSED AREA DOES NOT EXCEED 600 SQUARE FEET. CONCRETE SLAB ON GRADE SHALL BE BOUNDED BY CONTROL JOINTS AS SHOWN ON FOUNDATION PLAN. KEYED CONTROL JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING POURING, ALL OTHER JOINTS MAY BE SAW CUT.

NO FLY ASH ADDITIVES SHALL BE USED IN FLATWORK OR ARCHITECTURALLY EXPOSED CONCRETE.

FLY ASH - IF PERMITTED BY ARCHITECTURAL SPECIFICATIONS, SHALL BE LIMITED TO 15% OF CEMENTITIOUS MATERIALS AND SHALL HAVE A REPLACEMENT FACTOR OF 1.2 RELATIVE TO CEMENT REPLACED.

MASONRY:

HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM 900, GRADE N, TYPE 1, F_m = 1,350 PSI, RUNNING BOND, MORTAR TYPE S, 1,800 PSI. GROUT 2,000 PSI. ROD GROUT IN VERTICAL SPACES IMMEDIATELY AFTER POURING AND AGAIN ABOUT 5 MINUTES LATER. PROVIDE CLEANOUTS IF GROUT LIFT EXCEEDS 4'-0" IN BLOCK WALLS. MAXIMUM GROUT LIFT SHALL BE 8'-0". UNLESS NOTED OTHERWISE ON THE PLANS, PLACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUN OF WALL EXCEEDS 20'-0". CONTROL JOINTS SHALL NOT OCCUR AT WALL CORNERS, INTERSECTIONS, ENDS, WITHIN 2'-0" OF CONCENTRATED POINTS OF BEARING OR JAMBS, OR OVER OPENINGS UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID.

VERTICAL REINFORCING:

1 #5 IN CENTER OF GROUT AT CENTER OF WALL, CONTINUOUS FULL HEIGHT OF WALL AT ALL CORNERS, INTERSECTIONS, WALL ENDS, BEAM BEARINGS, JAMBS, EACH SIDE OF CONTROL JOINTS AND AT INTERVALS NOT TO EXCEED 48" O.C. UNLESS NOTED OTHERWISE. TIE AT 8'-0" VERTICALLY, WITH SINGLE WIRE LOOP TIE BY A.A. WIRE PRODUCTS COMPANY. LAP SPLICES SHALL BE 40 BAR DIAMETERS FOR UP TO #5 BARS AND 48 BAR DIAMETERS FOR #6 BARS AND LARGER. DOWEL ALL VERTICAL REINFORCING TO FOUNDATION WITH DOWELS TO MATCH VERTICAL WALL OR COLUMN REINFORCING.

HORIZONTAL REINFORCING:

2 #5 IN MINIMUM 8" DEEP GROUTED CONTINUOUS BOND BEAM AT ROOF AND ELEVATED FLOOR LINES. 1 #5 IN MINIMUM 8" DEEP GROUTED CONTINUOUS BOND BEAM AT TOP OF PARAPET OR TOP OF A FREESTANDING WALL. PLACE THESE BARS CONTINUOUS THRU CONTROL JOINTS. WRAP MASTIC TAPE FOR 1'-6" EACH SIDE OF CONTROL JOINT. PROVIDE BENT BARS PER TYPICAL DETAILS, TO MATCH HORIZONTAL BOND BEAM REINFORCING AT CORNERS AND WALL INTERSECTION TO MAINTAIN BOND BEAM CONTINUITY. LAP SPLICES SHALL BE 40 BAR DIAMETERS FOR UP TO #5 BARS AND 48 BAR DIAMETERS FOR #6 BARS AND LARGER. STAGGER SPLICES A MINIMUM OF 40 BAR DIAMETERS. DO NOT SPLICE WITHIN 8'-0" OF CONTROL JOINTS. STANDARD WEIGHT (NO. 9 GAGE WIRE) DUR-O-WAL OR DUR-O-WIRE LADDER TYPE JOINT REINFORCEMENT AT 16" O.C. IN MASONRY WALLS.

DOUBLE ANGLE LINTELS:

UNLESS NOTED OTHERWISE OR SHOWN, PROVIDE THE FOLLOWING LINTELS IN 8" NON-BEARING MASONRY WALLS: PROVIDE MINIMUM 5" BEARING OF ANGLES ON JAMBS. FOR BEARING WALLS SEE SKETCH WHERE THESE ANGLES MAY BE USED. (NOTE: WHERE THE REQUIREMENTS OF THIS SKETCH ARE NOT POSSIBLE, NOTIFY THE STRUCTURAL ENGINEER).

OPENING WIDTH	LINTEL ANGLES	OPENING WIDTH OR GREATER
0' - 3'-4"	2 - 3 1/2" x 2 1/2" x 1/4" (SLV)	
3'-5" - 4'-0"	2 - 3 1/2" x 3" x 1/4" (SLV)	
4'-1" - 6'-0"	2 - 3 1/2" x 3 1/2" x 1/4" (SLV)	
6'-1" OR LARGER	NOTIFY STRUCTURAL ENGINEER	

THESE LINTELS, OR THE OPENING THEY SPAN, SHALL NOT BE PLACED SO AS TO INTERFERE WITH THE REQUIREMENTS OF OTHER STRUCTURAL ELEMENTS (I.E. BOND BEAMS, LINTELS, CONTROL JOINTS, CONCENTRATED POINTS OF BEARING, ETC.) WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. SOLID GROUT SHALL BE PROVIDED BETWEEN WEBS AND MASONRY FACE SHELLS FOR FULL LENGTH OF ALL STEEL LINTELS. MORTAR MAY BE USED FOR GROUT FOR THIS PURPOSE ONLY. FACE UNITS, SOAPS, ROMANS, ETC., SHALL BE LAID WITH FULL HEAD AND BED JOINTS. FOR ADDITIONAL INFORMATION AT OPENINGS IN MASONRY WALLS, SEE TYPICAL DETAILS.

REINFORCING:

ASTM A615 (F_y = 60 KSI) DEFORMED BARS FOR ALL BARS #6 AND LARGER. ASTM A615 (F_y = 40 KSI) DEFORMED BARS FOR ALL BARS #5 AND SMALLER. NO TACK WELDING OF REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
EXPOSED TO EARTH OR WEATHER	2"
#6 OR LARGER	
#5 AND SMALLER	
ALL OTHER PER LATEST EDITION OF ACI 318.	1 1/2"

LAP SPLICES IN CONCRETE:

LAP SPLICES UNLESS NOTED OTHERWISE, IN CONCRETE SHALL BE CLASS "B" TENSION LAP SPLICES PER LATEST EDITION OF ACI 318. STAGGER SPLICES A MINIMUM OF ONE LAP LENGTH.

ALL SPLICE LOCATIONS SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT ALL CORNERS AND INTERSECTIONS PER TYPICAL DETAILS. REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. DOWEL ALL VERTICAL REINFORCING TO FOUNDATION, SECURELY-TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE.

DRYPACK

DRYPACK SHALL BE 5,000 PSI FIVE STAR NON-SHRINK GROUT OR EQUIVALENT. DRYPACK UNDER BEARING PLATES BEFORE FRAMING MEMBER IS INSTALLED. AT COLUMNS, INSTALL DRYPACK UNDER BASEPLATES AFTER COLUMN HAS BEEN PLUMBED BUT PRIOR TO FLOOR OR ROOF INSTALLATION.

STRUCTURAL STEEL

ALL STRUCTURAL STEEL SHALL BE ASTM A36 (F_y = 36 KSI). ALL PIPE STEEL SHALL BE ASTM A501 (F_y = 36 KSI) OR ASTM A53, TYPE E OR S, GRADE B (F_y = 35 KSI). ALL TUBULAR STEEL SHALL BE ASTM A500 (F_y = 46 KSI). ALL BOLTS SHALL BE ASTM A307, UNLESS NOTED OTHERWISE. ALL CONSTRUCTION PER LATEST AISC HANDBOOK. ALL EXPANSION BOLTS TO HAVE I.C.B.O. RATING FOR MATERIAL INTO WHICH INSTALLATION TAKES PLACE. ALL BOLTS, ANCHOR BOLTS, EXPANSION BOLTS, ETC. SHALL BE INSTALLED WITH STEEL WASHERS AT FACE OF WOOD OR AT SLOTTED HOLES IN STEEL SECTIONS. ALL WELDS SHALL BE BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS. FOR ASTM A615-GRADE 60 REINFORCING BARS, USE E90 SERIES. ALL WELDING PER LATEST AMERICAN WELDING SOCIETY STANDARDS. ALL WELDS ON DRAWINGS ARE SHOWN AS SHOP WELDS. CONTRACTOR MAY SHOP WELD OR FIELD WELD AT HIS DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW.

PREFABRICATED WOOD TRUSSES:

PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED TO SUPPORT SELF WEIGHT PLUS LIVE LOAD AND SUPERIMPOSED DEAD LOADS STATED IN THE GENERAL STRUCTURAL NOTES OR LOCATED ON PLANS. BRIDGING SIZE AND SPACING BY TRUSS MANUFACTURER UNLESS NOTED OTHERWISE. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, ERECTION DRAWINGS AND DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER FOR REVIEW PRIOR TO MANUFACTURE. CALCULATIONS AND SHOP DRAWINGS SHALL SHOW ANY SPECIAL DETAILS REQUIRED AT BEARING POINTS. ALL CONNECTORS SHALL HAVE CURRENT I.C.B.O. APPROVAL.

ADDITIONAL TRUSSES SHALL BE SUPPLIED AS REQUIRED TO SUPPORT MECHANICAL EQUIPMENT.

MULTIPLE TRUSS MEMBERS SHALL BE FASTENED TOGETHER TO ALLOW TRANSFER OF SHEAR AND TENSION FORCES (MINIMUM 200 PLF) AT PLYWOOD SHEATHING JOINTS AND TO PREVENT CROSS GRAIN BEARING OF TOP CHORDS. ATTACHMENT SHALL BE A CONTINUOUS 2" GAGE METAL PLATE OR OTHER APPROVED MEANS. METHOD OF ATTACHMENT SHALL BE INDICATED ON SHOP DRAWINGS FOR REVIEW.

WOOD:

SAWN LUMBER:

FRAMING LUMBER SHALL COMPLY WITH THE LATEST EDITION OF THE GRADING RULES OF THE WESTERN WOOD PRODUCTS ASSOCIATION OR THE WEST COAST LUMBER INSPECTION BUREAU. ALL SAWN LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED LUMBER GRADING AGENCY. JOISTS, LEDGERS, BEAMS, ETC. SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

	F _b (PSI)	F _v (PSI)	E (PSI)	F _c (PSI)	WOOD TYPE
JOISTS					
2 X 4	1,650 (REP)	95	1,700,000	N/A	D.F. #2
2 X 6 OR LARGER	1,450 (REP)	95	1,700,000	N/A	D.F. #2
BEAMS					
WIDTH 4" OR LESS	1,500 (SING)	95	1,800,000	N/A	D.F. #1
LEDGERS AND TOP PLATES - 1,250 (SING)	95	1,700,000	N/A	D.F. #2	
STUDS					
2 X 4	675 (REP)	95	1,500,000	925	D.F. STD.
2 X 6 OR LARGER	1,450 (REP)	95	1,700,000	1,050	D.F. #2
4 X 4	1,450 (SING)	95	1,700,000	1,000	D.F. #2

GLUED-LAMINATED BEAMS (GLULAM):

GLUED-LAMINATED BEAMS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: F_b = 2,400 PSI, F_v = 195 PSI, F_c (PERPENDICULAR) = 650 PSI, E = 1,700,000 PSI. BEAMS CANTILEVERING OVER SUPPORTS SHALL HAVE THE SPECIFIED MINIMUM PROPERTIES TOP AND BOTTOM. ALL BEAMS SHALL BE FABRICATED USING WATERPROOF GLUE. FABRICATION AND HANDLING PER LATEST AITC AND WCLA STANDARDS. BEAMS TO BEAR GRADE STAMP AND AITC STAMP AND CERTIFICATE. CAMBER AS SHOWN ON DRAWINGS.

PLYWOOD:

ALL PLYWOOD SHALL BE C-D INTERIOR SHEATHING WITH EXTERIOR GLUE AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY. LAY UP PLYWOOD WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. (ON ROOFS WHERE PLYWOOD IS LAYED UP WITH FACE GRAIN PARALLEL TO SUPPORTS, USE A MINIMUM OF 5-PLY PLYWOOD). STAGGER JOINTS. ALL NAILING, COMMON NAILS. ALL PLYWOOD SHALL BE OF THE FOLLOWING NOMINAL THICKNESS, SPAN/INDEX RATIO AND SHALL BE NAILED AS FOLLOWS UNLESS NOTED OTHERWISE:

USE	THICKNESS	SPAN/INDEX RATIO	EDGE NAILING	INTERMEDIATE NAILING
ROOF	5/8"	42/20	10d @ 6" O.C.	10d @ 12" O.C.

GENERAL:

DO NOT NOTCH OR DRILL JOISTS, BEAMS OR LOAD BEARING STUDS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER THRU THE ARCHITECT. DOUBLE UP FLOOR JOISTS AND BLOCKING UNDER PARTITIONS. PROVIDE 1 X 3 OR METAL CROSS BRIDGING AT MIDSPAN AT ALL JOISTS. PROVIDE 2" SOLID BLOCKING AT SUPPORTS OF ALL JOISTS. DOUBLE UP STUDS AT JAMBS, AND AS REQUIRED UNDER BEAMS, IN BEARING WALLS. PROVIDE 2 X SOLID BLOCKING AT MID-HEIGHT OF BEARING STUD WALLS. ALL NAILING NOT NOTED SHALL BE ACCORDING TO TABLE 250 OF THE UNIFORM BUILDING CODE. JOIST HANGERS AND OTHER MISCELLANEOUS FRAMING ANCHORS SHALL BE AS MANUFACTURED BY SIMPSON COMPANY OR OTHER MANUFACTURER WITH CURRENT I.C.B.O. APPROVAL.

SHOP DRAWINGS:

SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS.

THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN ACCORDANCE WITH CONTRACT DRAWINGS SHALL BE FLAGGED UPON HIS REVIEW. VERIFY ALL DIMENSIONS WITH ARCHITECT.

ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM ORIGINAL CONTRACT DRAWINGS SHALL BE CLOUDED BY MANUFACTURER OR FABRICATOR. ANY OF THE AFORESAID WHICH ARE NOT CLOUDED OR FLAGGED BY SUBMITTING PARTIES, SHALL NOT BE CONSIDERED APPROVED AFTER ENGINEERS REVIEW, UNLESS NOTED ACCORDINGLY.

THE ENGINEER HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO ORIGINAL DRAWINGS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW.

THE SHOP DRAWINGS DO NOT REPLACE THE ORIGINAL CONTRACT DRAWINGS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT ARE NOT TO BE CONSIDERED CHANGES TO ORIGINAL CONTRACT DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAKE SURE ITEMS ARE CONSTRUCTED TO ORIGINAL DRAWINGS.

THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY THE OTHERS RESTS WITH THE DESIGNING OR SUBMITTING AUTHORITY.

REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR.

GENERAL:

THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS NOR WILL THE STRUCTURAL ENGINEER BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO.

CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA.

ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL WITH APPROPRIATE TRADES, DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION.

OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF HE CHOOSES AN OPTION, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES AND SHALL COORDINATE ALL DETAILS.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT.

TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.

ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF ARIZONA.

ABBREVIATIONS:

A.B.C.	AGGREGATE BASE COURSE	I.F.W.	INSIDE FACE OF WALL
A/C	AIR CONDITIONER	HORIZ	HORIZONTAL
A.F.F.	ABOVE FINISHED FLOOR	LBS (#)	POUNDS
ALT.	ALTERNATE	LLV	LONG LEG VERTICAL
A.L.B.	ANCHOR BOLT	M.B.S.	MACHINE BOLT
ARCH'L	ARCHITECTURAL	MFR(S)	MANUFACTURER(S)
#	AT (MEASUREMENT)	MAS C.J.	MASONRY CONTROL JOINT
BM	BEAM	MAX	MAXIMUM
B.O.B.	BOTTOM OF BEAM	MECH'L	MECHANICAL
B.O.D.	BOTTOM OF DECK	MIN	MINIMUM
BRG	BEARING	N/A	NOT APPLICABLE
C.I.P.	CAST IN PLACE	N.T.S.	NOT TO SCALE
C.L.	CENTERLINE	N.O. (#)	NUMBER
C.L.B.	CENTERLINE OF BEAM	O.C.	ON CENTER
C.L.C.	CENTERLINE OF COLUMN	O.F.W.	OUTSIDE FACE OF WALL
C.L.W.	CENTERLINE OF WALL	OP	OPPOSITE
CONC	CONCRETE	P.C.	PRECAST CONCRETE
CONC C.J.	CONCRETE CONTROL JOINT	PLF	POUNDS LINEAR FOOT
CONC S.J.	CONCRETE SAWCUT JOINT	PREFAB	PREFABRICATED
C.M.U.	CONCRETE MASONRY UNIT	PSF	POUNDS SQUARE FOOT
CONN	CONNECTION	PSI	POUNDS SQUARE INCH
CONT	CONTINUOUS	REINF	REINFORCING
Ø OR DIA	DIAMETER	SLV	SHORT LEG VERTICAL
DN	DOWN	SHM	SIMILAR
DWS(S)	DRAWING(S)	SQ.	SQUARE
EQ	EQUAL	STD	STANDARD
EQUIP	EQUIPMENT	STRUCT'L	STRUCTURAL
ETC	ETCETERA	T.O.B.	TOP OF BEAM
EXP JT (E.J.)	EXPANSION JOINT	T.O.D.	TOP OF DECK
E.W.	EACH WAY	T.O.F.	TOP OF FOOTING
F.F.	FINISHED FLOOR	T.O.L.	TOP OF LEDGER
F.O.S.	FACE OF STEEL	T.O.S.	TOP OF STEEL
F.O.W.	FACE OF WALL	T.O.W.	TOP OF WALL
GA	GAGE	TS	TUBE STEEL
GALV	GALVANIZED	TYP	TYPICAL
G.S.N.	GENERAL STRUCTURAL NOTES	U.N.D.	UNLESS NOTED OTHERWISE
GLB (GLULAM)	GLUED-LAMINATED BEAM	VERT	VERTICAL
		W.F.	WELDED WIRE FABRIC
		W/	WITH

INTERPRETATION OF DRAWINGS

LOCATION OF INFORMATION			
ITEM	INFORMATION	LOCATION	SHEET
FOOTINGS	SIZE, REINFORCING	SCHEDULE (F) (WF)	S2.1
	DEPTH OF FOOTING	GENERAL STRUCTURAL NOTES (G.S.N.)	S1.1
FRAMING MEMBERS	TYPE, SIZE, CONNECTION CAMBER, BEARING PLATES	SCHEDULE (B) (T) (SL)	S2.2
COLUMNS	TYPE, SIZE, BASE PLATES, REINFORCING	SCHEDULE (C)	S2.1
MASONRY WALLS	TYPICAL REINFORCING SPECIAL REINFORCING	GENERAL STRUCTURAL NOTES (G.S.N.) SEE PLAN(S) AND/OR DETAILS	S1.1 S2.1

PLAN LEGEND		
SYMBOL	DESCRIPTION	REMARKS
	DETAIL CUTS SHOWN ON PLAN	TYPICAL DETAILS ARE 01 SERIES NUMBERS - S1.2 FOUNDATION DETAILS ARE 100 SERIES NUMBERS - S3.1 FRAMING DETAILS ARE 200 SERIES NUMBERS - S4.1
	8" MASONRY WALL U.N.O.	OTHER SIZES ARE DIMENSIONED ON PLANS
	MECHANICAL EQUIPMENT	SEE PLANS FOR UNIT WEIGHTS
	OPENING IN FRAMING PLAN	SEE NOTE 4.

NOTES	
1.	FOR MATERIAL STRENGTHS, SEE GENERAL STRUCTURAL NOTES.
2.	VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION - RESOLVE ANY DISCREPANCY WITH ARCHITECT.
3.	FOR CLARITY, ALL EXTERIOR SLABS AND SIDEWALKS MAY NOT BE SHOWN. FOR EXACT DIMENSIONS, LOCATIONS, JOINT AND SCORE LINES, SEE ARCHITECTURAL DRAWINGS.
4.	FOR CLARITY, ALL OPENINGS MAY NOT BE SHOWN ON FRAMING PLANS. FOR EXACT SIZE, NUMBER, AND LOCATION OF OPENINGS, SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. FOR FRAMING AT OPENINGS, SEE TYPICAL STRUCTURAL DETAILS. VERIFY ALL SIZES, WEIGHT AND LOCATIONS OF MECHANICAL EQUIPMENT WITH MECHANICAL ENGINEER & MECHANICAL CONTRACTOR THROUGH ARCHITECT.
5.	DETAILS MARKED "TYPICAL" MAY NOT BE CUT ON PLANS.
6.	CONC C.J. - AS SHOWN ON PLAN INDICATES LOCATION OF EITHER KEYED OR SAW CUT CONTROL JOINT IN SLAB ON GRADE AT CONTRACTOR'S OPTION, SEE GENERAL STRUCTURE NOTES & PLANS.
7.	MAS. C.J. - AS SHOWN ON PLAN INDICATES MASONRY CONTROL JOINT IN MASONRY WALL, SEE G.S.N. AND TYPICAL DETAIL.
8.	FOR CLARITY, DETAILS MAY SHOW ONLY ONE SIDE OF FRAMING CONDITION.

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MeraBank
 KINGMAN, ARIZONA
 BRANCH BANK

JOB NUMBER
10434.00
 DATE
11-11-88
 DRAWN
WLC
 CHECKED
CRP
 SHEET TITLE
G.S.N. & I.O.D.
 SCALE
NONE
 REVISIONS

	88-581 Project Number	For additional
	CRP Project Manager	information shown,
	CRP Project Engineer	but not noted.
	WLC Project Drafter	see sheet S1.1

DRAWING NUMBER
S1.1