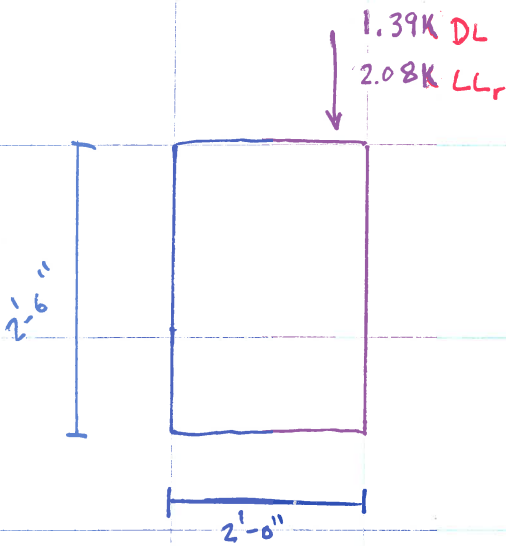
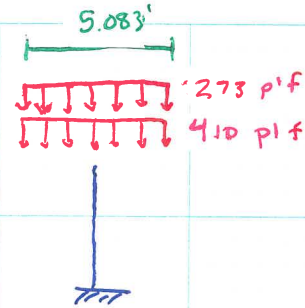




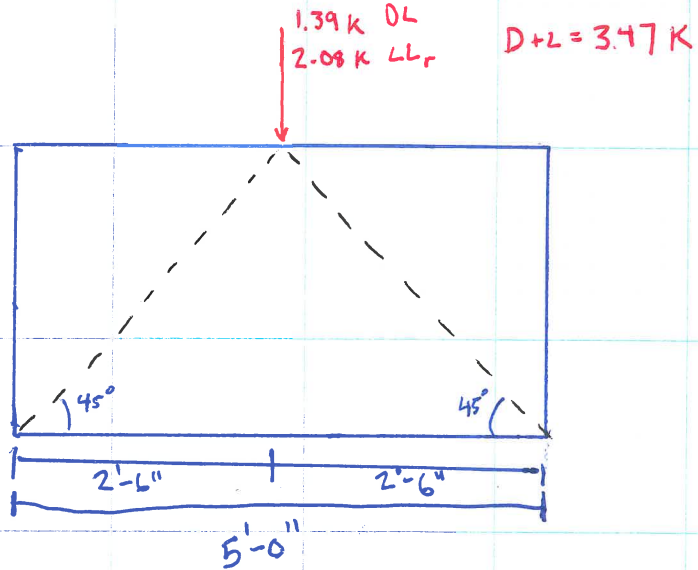
WALL FTG UNDER POST @ SIDE WINDOWS

TRIBUTARY WIDTH = $5'-1" = 5.083'$
 $OL = 20 \text{ PSF} \cdot \frac{27.33'}{2} = 273.4 \text{ plf}$
 $LL_r = 30 \text{ PSF} \cdot \frac{27.33'}{2} = 410.1 \text{ plf}$

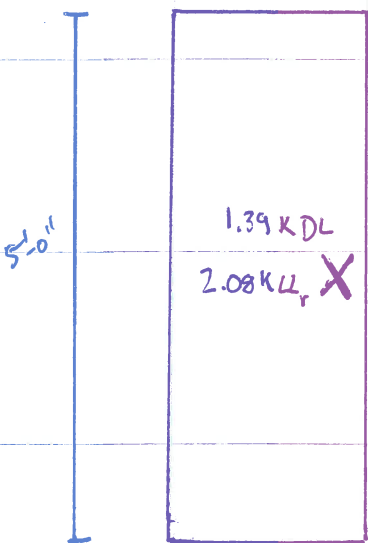
$DL = 273.4 \cdot 5.083 = 1.39 \text{ K}$
 $LL_r = 410.1 \cdot 5.083 = 2.08 \text{ K}$



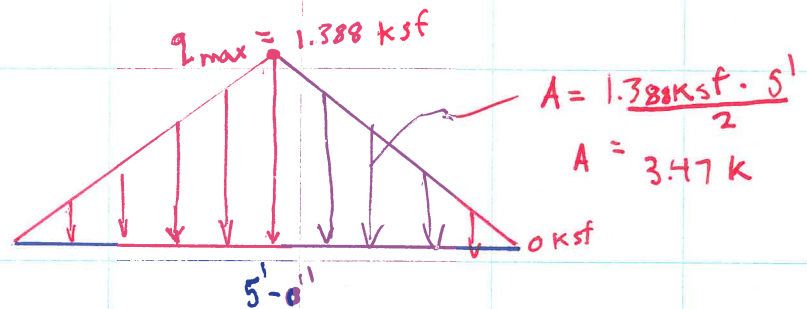
SECTION



ELEVATION



PLAN



PRESSURE DISTRIBUTION
ALONG BOTTOM OF FOOTING

$1.388 \text{ ksf} < 2.5 \text{ ksf} \therefore \frac{OK}{=}$

Britt, Peters & Associates

101 W. Camperdown Way, Suite 601
Greenville, SC 29601

Job Name: TB - Takoma Park, MD

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Client:

By: DRS

Job No.: 150699

Date: 4/5/16

Embedded Post Foundation Design

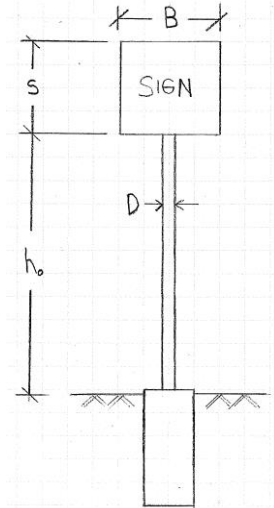
Wind Criteria:	V	Exposure	K_{zt}	K_d	I	G	K_z	q_z
	115 mph	B	1	0.85	1	0.85	0.69	19.8 psf

Pole Geometry

Pole Shape:	Square
Side length, D =	5 in
Height above grade, h_o =	28 ft
h_o/D =	67.2
C_f =	1.5
F pole(ultimate) =	416 lb applied at 14 ft above grade
F pole(service) =	250

Sign Geometry

Sign height, s =	1 ft
Sign width, B =	2 ft
C_f =	1.95
F sign(Ultimate) =	66 lb applied at 28.5 ft above grade
F sign(Service) =	39



Equivalent Force for Foundation Design

F =	289 lb
h =	26.6 ft above grade

Use wind load from above

Loading

P =	0 lbs
h =	0 ft
M_g =	7689.4 lb-ft
P_{EQ} =	288.8 lbs
h_{EQ} =	26.6 ft

Soil

Allowable Bearing	2000 psf/f
Lateral Bearing	150 psf/f

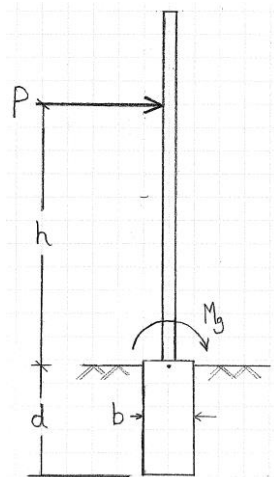
- Pole is not adversely affected by a 0.5 in. deflection
- Slab/Pavement is present to constrain Pole/Post at ground level

S = 300 psf

Foundation

Shape:	Round
Diameter, b =	30 in
d =	72 in
$d_{REQ'D}$ =	67.0 in

Use a 30 in diameter footing with a 72 in embedment



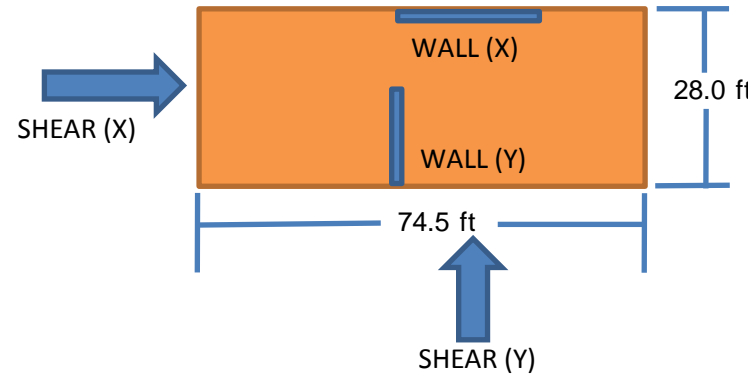
BRITT, PETERS & ASSOCIATES, INC.
CONSULTING ENGINEERS
(864) 271-8869 FAX (864) 233-5140
101 Falls Park Drive, Suite 601, Greenville SC 29601

Subject: Taco Bell - Takoma, MD
Project No.: 150699
By: DRS
Date: 10/19/2015

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Shear Wall Design Spreadsheet

Base Shears		Code: ASCE 7-10
Ultimate Design Loads	Service Loads	Seismic Design Category: B
Wind Y = 40.0 kips	24 kips	Wood Species: SP
Wind X = 15.0 kips	9 kips	Nails: 10d
Seismic = 4.0 kips	3 kips	Wall DL (psf) 10.0
		Roof DL (psf) 20.0
Controlling Case Y-direction: 24 kips	Wood Species Factor = 1	Sill Anch. Dia.: 5/8
Controlling Case X-direction: 9 kips		



Wind Controls in Y Direction
Wind Controls in X Direction

X-Direction Shear Walls

SW#	Location	Trib Width	Length (ft)	Wall Height (ft)	Aspect Ratio	Perforated SW	Fwall (kips)	Shear(plf)	V/1.4 (Wind)	Use V (plf)	15/32" Plywood		DBL		X-Direction Shear Walls				Holddown				Anchor Bolts			
											Nail	Spacing	Sheathing Nail	Sheathing Spacing	Tension(k)	Roof Trib (ft)	Wall/Roof DL (plf)	Segment Length	Tnet (kips)	Shear (plf)	Sill Thickness (in)	Type	Anchor Dia.	Diameter	Spacing	
1	Drive-thru	14.00	41.50	13.17	0.32	NO	4.5	108	77	77	10d	6	Not Used	6	1.4	14.00	412	41.50	3.7	77	1.5"	NR	NG	5/8	172	
1	Dining	14.00	12.25	14.00	1.14	NO	4.5	367	262	262	10d	6	Not Used	6	5.1	14.00	420	12.25	-3.6	262	1.5"	HDU4	5/8	5/8	51	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.00	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	

28.0 ft

Y-Direction Shear Walls

SW#	Location	Trib Width	Length (ft)	Wall Height (ft)	Aspect Ratio	Perforated SW	Fwall (Kips)	Shear(plf)	V/1.4 (Wind)	Use V (plf)	15/32" Plywood		DBL		Y-Direction Shear Walls				Holddown				Anchor Bolts			
											Nail	Spacing	Sheathing Nail	Sheathing Spacing	Tension(k)	Roof Trib (ft)	Wall/Roof DL (plf)	Segment Length	Tnet (kips)	Shear (plf)	Sill Thickness (in)	Type	Anchor Dia.	Diameter	Spacing	
2	Tower	12.50	5.50	14.00	2.55	NO	4.0	732	523	523	10d	3	10d B.S.	6	10.3	1.000	160	5.50	-10.0	523	1.5"	HDU14	5/8	5/8	25	
3	Interior	37.25	8.17	10.67	1.31	NO	12.0	1469	1050	1050	10d	BS	10d B.S.	3	15.7	1.000	127	8.17	-15.368	1050	1.5"	HD19	5/8	5/8	13	
2	Rear Wall	24.75	26.00	14.00	0.54	NO	8.0	307	219	219	10d	6	Not Used	6	4.3	1.000	160	26.00	-3.0	219	#DIV/0!	HDU2	5/8	5/8	61	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.000	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.000	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.000	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.000	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.000	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	
						NO	0.0	#DIV/0!	#DIV/0!	#DIV/0!	10d	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	1.000	20	0.00	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	5/8	#DIV/0!	

74.5 ft

Shear Wall End Holddown and Sill Anchorage

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Subject: Taco Bell - Takoma, MD
 Project No.: 150699
 By: DRS
 Date: 10/19/2015

Shear Wall Collectors and Diaphragm Blocking

Nominal Framing Width: 2"
 Sheathing and Fastening Used: 19/32" PS1/PS2 10d

X-Direction Shear Walls Diaphragm Load Case: Case 3

SW#	Location	Trib Width	Segment Length	Shear(plf)	Force	Blocking & Nailing	Diaphragm Capacity (plf)	Collector Force (lb)	Collector Length (ft)	Collector Required	Collector
1	Drive-thru	14.00	41.50	77	4.5	Blocked 4,6,12	425	0	0	NO	
1	Dining	14.00	12.25	262	4.5	Blocked 4,6,12	425	0	0	NO	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	425	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.00	0.00	#DIV/0!	0.0	Unblocked 6,12	215	#DIV/0!	#DIV/0!	#DIV/0!	
		28.0 ft									

Y-Direction Shear Walls Diaphragm Load Case: Case 1

SW#	Location	Trib Width	Segment Length	Shear(plf)	Force	Blocking & Nailing	Diaphragm Capacity (plf)	Collector Force (lb)	Collector Length (ft)	Collector Required	Collector
2	Tower	12.5	5.5	523	4.0	Blocked 4,6,12	425	539	1	YES	2x6 Ledger at front wall will collect load
3	Interior	37.3	8.2	1050	12.0	Blocked 4,6,12	425	5101	12	YES	Drag Truss
2	Rear Wall	24.8	26.0	219	8.0	Blocked 4,6,12	425	0	0	NO	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
0	0	0.0	0.0	#DIV/0!	0.0	Unblocked 6,12	285	#DIV/0!	#DIV/0!	#DIV/0!	
		74.5 ft									

$q'_a = 2.5$ ksf
 Allowable increase 1
 input L = 14.5 ft
 input B = 4 ft
 x offset = 0 ft
 y offset = 0 ft
 footing thickness 30 in
 Depth of Footing 0.00001 ft
 Soil Density 120 psf
 Slab Thickness 4 in
 Slab Extension 6.62 ft
 Soil Friction Angle 45 degrees

$q_{ult} = 2.8$ ksf
 $q'_{max} = 0.87$ ksf
 Dead load factor for footin and soil wt. = 0.6
 Edge "E", Interior "I", or Corner "C" I

Suggested L = ft
 Suggested B = ft

	Dim	Area
SQ =	8.50	72.25
Rect B =	4.00	58
Rect L =	14.50	

Largest Rect. Area
 21.75
 Largest Square Area
 72.25



	P(k)	Mx'(kft)	My'(kft)	P+Footing	Mx(kft)	My(kft)	eL(ft)	eB(ft)
3	0.6	91.5		25.9	91.5	0	3.537	0.000

Rectangular Footing Per Load Case						
OR	Suggested L =	A2(ft^2)	L	B	L'	B'
1	10.5	22	14.5	1.50	7.425	1.240

Optimal Shape
 RECTANGULAR

Rectangular Footing Check For All Cases				
L'(ft)	B'(ft)	A'(ft)	q'equiv	
7.4	4.0	29.7	0.9	OK

Project: TB - Takoma Park, MD
 BPA No.: 150699
 Date: 10/19/2015
 By: DRS

Description:
 Interior Shear Wall Footing

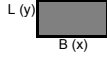
$q'_a = 2.5$ ksf
 Allowable increase 1
 input L = 27 ft
 input B = 2 ft
 x offset = 0 ft
 y offset = 7 ft
 footing thickness 30 in
 Depth of Footing 0.00001 ft
 Soil Density 120 psf
 Slab Thickness 4 in
 Slab Extension 6.62 ft
 Soil Friction Angle 45 degrees

$q_{ult} = 2.8$ ksf
 $q'_{max} = 0.46$ ksf
 Dead load factor for footing and soil wt. = 0.6
 Edge "E", Interior "I", or Corner "C" E

Suggested L = ft
 Suggested B = ft

	Dim	Area
SQ =	6.00	36.00
Rect B =	2.00	54
Rect L =	27.00	

Largest Rect. Area 13.50
 Largest Square Area 36.00



	P(k)	Mx'(kft)	My'(kft)	P+Footing	Mx(kft)	My(kft)	eL(ft)	eB(ft)
3	0.5	40.3		20.8	43.8	0	2.109	0.000

Rectangular Footing Per Load Case						
OR	Suggested L =	A2(ft^2)	L	B	L'	B'
1	7	14	27	0.50	22.782	0.328

Optimal Shape
 RECTANGULAR

Project: TB - Takoma Park, MD
 BPA No.: 150699
 Date: 10/19/2015
 By: DRS

Description:
 Front Tower Shear Wall Footing

Rectangular Footing Check For All Cases				
L'(ft)	B'(ft)	A'(ft)	q'equiv	
22.8	2.0	45.6	0.5	OK