GENERAL NOTES

- A. BUILDING AND DESIGN CODES:
- 1. 2008 INDIANA BUILDING CODE.
- 2. ASCE 7-05 MINIMUM DESIGN LOADS BUILDING AND OTHER STRUCTURES.
- 3. AFPA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION.
- 4. ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, 2011.

B. DESIGN LOADS:

ENGINEERING DATA

FLOOR LIVE LOADS: 50 PSF ROOF LIVE LOAD: 20 PSF

LIVE LOAD: 20 PSF

LIVE LOAD REDUCTION ON SUPPORTING ELEMENTS IN ACCORDANCE WITH 2006 IBC.

LOADING FOR MECHANICAL AND ELECTRICAL ROOMS ARE BASED ON THE WEIGHTS OF ASSUMED EQUIPMENT, AS INDICATED ON THE MECHANICAL DRAWINGS (INCLUDING THE WEIGHT OF CONCRETE PADS WHERE INDICATED ON THE MECHANICAL DRAWINGS). ANY CHANGES IN TYPE, SIZE, LOCATION, OR NUMBER OF PIECES OF EQUIPMENT SHOULD BE REPORTED TO THE ENGINEER FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO PLACEMENT OF EQUIPMENT.

ROOF DEAD LOAD: 18 PSF
DESIGN DEAD LOADS INCLUDE THE WEIGHT OF STRUCTURAL COMPONENTS AND PERMANENT FIXTURES.

GROUND SNOW LOAD: Pg = 20 PSF FLAT ROOF SNOW LOAD: Pf = 20 PSF SNOW EXPOSURE FACTOR: Ce = 1.0

SNOW EXPOSURE FACTOR: Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR: I = 1.0 THERMAL FACTOR: Ct = 1.1

WIND DESIGN DATA:
BASIC WIND SPEED (3 SECOND GUST): 90 MPH
WIND IMPORTANCE FACTOR: = 1.0
WIND EXPOSURE CATEGORY: C

BUILDING CATEGORY: II — ENCLOSED BUILDING

INTERNAL PRESSURE COEFFICIENTS: ±0.18
ALL NEW COMPONENTS AND CLADDING NOT DESIGNED BY THE ENGINEER SHALL BE DESIGNED FOR 25 PSF UNLESS OTHERWISE APPROVED BY THE ENGINEER

DESIGN UPLIFT - 23.7 PSF

SEISMIC IMPORTANCE FACTOR, I: 1.0
MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss = 0.152 S1 = 0.071

SITE CLASS: D SPECTRAL RESPONSE COEFFICIENTS - Sds = 0.162, Sd1 = 0.113

SEISMIC DESIGN CATEGORY: B
BASIC SEISMIC-FORCE-RESISTING SYSTEM: BEARING WALL SYSTEM, LIGHT FRAMED WALLS WITH

DESIGN BASE SHEAR: V = 2.79 kips SEISMIC RESPONSE COEFFICIENT: Cs = 0.027 RESPONSE MODIFICATION FACTOR: R = 6.5 ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE METHOD

FLOOD HAZARD INFORMATION: THIS BUILDING IS NOT DESIGNED FOR FLOOD LOADS.

C. GENERAL REQUIREMENTS:

1. SPECIFICATIONS ARE PART OF THE CONSTRUCTION DOCUMENTS AND MUST BE USED IN CONJUNCTION WITH THE DRAWINGS.

- 2. VERIFY THE LOCATION OF CHASES, INSERTS, OPENINGS, SLEEVES, FINISHES, DEPRESSIONS, PADS, AND WALL OPENINGS.
- 3. DO NOT SCALE DRAWINGS FOR THE PURPOSE OF ESTABLISHING DIMENSIONS.
- 4. DETAILS LABELED "TYPICAL DETAILS" ON DRAWINGS APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT DETAILS ARE REFERENCED AT EACH LOCATION. NOTIFY ENGINEER OF CONFLICTS REGARDING APPLICABILITY OF "TYPICAL DETAILS".
- 5. DO NOT STORE OR STACK CONSTRUCTION MATERIALS ON POURED OR ERECTED FLOORS/ROOFS IN EXCESS OF 80 PERCENT OF LIVE LOAD. GENERAL CONTRACTOR WILL ENSURE THAT ALL SUB—CONTRACTORS ARE INFORMED OF LOADING RESTRICTIONS. AVOID IMPACT WHEN PLACING MATERIALS ON POURED OR ERECTED FLOORS OR ROOF.
- 6. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL MEASURES REQUIRED TO PROTECT THE STRUCTURE, WORKMEN, AND OTHER PERSONS DURING CONSTRUCTION; INCLUDING BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, FORMS AND SCAFFOLDING, SHORING OF RETAINING WALLS AND OTHER TEMPORARY SUPPORTS AS REQUIRED.
- PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON DRAWINGS. EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS
 FOR THE REQUIRED OPENINGS AND PROVIDE FOR REQUIRED OPENINGS WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT, VERIFY
 SIZE AND LOCATION OF OPENINGS WITH THE MECHANICAL CONTRACTOR. DEVIATIONS FROM THE OPENINGS SHOWN ON THE STRUCTURAL
 DRAWINGS MUST BE APPROVED PRIOR TO IMPLEMENTING THE CHANGES.

FOUNDATION NOTES

1. FLOOR DRAIN GRATES SHALL BE FLUSH WITH FINISHED FLOOR ALL FLOOR DRAINS TO BE 1/2" BELOW FINISHED FLOOR.

- 2. SLOPE FLOOR 1/16" PER FOOT MIN. AT ALL FLOOR DRAINS AND FLOOR SINKS AS SHOWN.
- 3. ALL SIMPSON HOLD DOWNS TO BE STAINLESS STEEL OR Z-MAX G185 HOT DIPPED GALVANIZED COATING.
- 4. UNLESS OTHEREWISE INDICATED BY SHEAR SCHEDULE, 1/2" X 10" SILL PLATE ANCHOR BOLTS WITH .229"x3"x3" PLATE WASHERS ARE TO PROVIDED AT 48" AT ALL SILL PLATES. ALL SILL PLATE ANCHOR BOLTS TO BE STAINLESS STEEL OR G185 HOT DIPPED GALVANIZED.
- 5. ALL STEEL IN CONTACT WITH TREATED WOOD TO BE STAINLESS STEEL OR G185 HOT DIPPED GALVANIZED.

SOIL PREPARATION NOTES

- 1. THE FOUNDATION DESIGN IS PRELIMINARY AND IS TO BE VERIFIED WITH THE FINAL GEOTECHNICAL REPORT PRIOR TO CONSTRUCTION.
- 2. PROVIDE POSITIVE DRAINAGE FOR ALL TRENCHES DURING CONSTRUCTION. DO NOT ALLOW ANY PONDING OF WATER DURING CONSTRUCTION
- 3. THE SOIL BENEATH THE BUILDING AND 5 FEET AROUND THE PERIMETER SHALL BE TREATED AS FOLLOWS:
- A. STRIP THE AREA OF ALL VEGETATION
 B. PROOF ROLL THE SITE WITH A TANDEM AXLE LOADED DUMP TRUCK IN TWO DIRECTIONS. ANY AREAS WHICH ARE NOTED TO RUT OR PUMP EXCESSIVELY SHALL BE UNDERCUT AND BACKFILLED WITH COMPACTED ON SITE SOIL OR SELECT FILL ACCORDING TO THE COMPACTED
- REQUIREMENTS NOTED BELOW.

 C. THE NEXT 8 INCHES SHALL BE THOROUGHLY SCARIFIED, WITH WATER ADDED TO RAISE THE MOISTURE CONTENT TO WITHIN 2 PERCENTAGE POINTS OF OPTIMUM AND RE-COMPACTED TO A DENSITY IN THE RANGE OF 95% TO 100% OF STANDARD PROCTOR. THE FIRST LIFT SHALL BE PLACED ON
- OF OPTIMUM AND RE-COMPACTED TO A DENSITY IN THE RANGE OF 95% TO 100% OF STANDARD PROCTOR. THE FIRST LIFT SHALL BE PLACED ON THE COMPACTED SUBGRADE WITHIN EIGHT HOURS OF COMPLETING THE COMPACTION.

 D. THE FILL REQUIRED TO RAISE THE BUILDING TO BENEATH THE FLOOR SLAB SHALL BE EITHER ON SITE FILL OR SELECT FILL. THE SELECT FILL SHALL HAVE A PLASTICITY INDEX BETWEEN 5 AND 15 AND A LIQUID LIMIT LESS THAN 30. PLACE THE FILL IN 8 INCH LIFTS AND COMPACT TO AT

LEAST 95% OF THE STANDARD PROCTOR. ANY AREAS OF LOOSE FILL OR DELETERIOUS MATERIALS SHALL BE REMOVED AND FILLED AS NOTED.

4. THE FOUNDATION DESIGN DOES NOT APPLY TO SITES ON EXPANSIVE SOIL, LOCATIONS WITH A WATER TABLE WITHIN 5 FEET OF THE SURFACE, OR AREAS WITH SOIL WHERE THE STRENGTH IS QUESTIONABLE, SUCH AS FILL (AS REQ'D IN IBC SECTION 1802.2). EFFORT HAS BEEN MADE TO VERIFY THAT THE SUPPORTING SOILS ARE ADEQUATE. IF CONDITIONS SUCH AS THOSE MENTIONED ABOVE EXIST, CONTACT THE ENGINEER.

SLAB-ON-GRADE NOTES

- A. SLAB ON GRADE PER 1/S2.01.
- B. THE SLAB SHALL BE UNDERLAIN BY A MINIMUM 10-MIL "STEGO WRAP" CLASS A VAPOR RETARDER BY STEGO INDUSTRIES OR VAPOR BLOCK 10 BY RAVEN INDUSTRIES TO BE PLACED BELOW THE FLOOR SLAB WHERE REQUIRED TO PROTECT MOISTURE—SENSITIVE FLOOR COVERINGS (I.E. TILE, OR CARPET, ETC.). THE SHEETS OF THE VAPOR RETARDER MATERIAL SHALL BE EVALUATED FOR HOLES AND/OR PUNCTURES PRIOR TO PLACEMENT AND THE EDGES OVERLAPPED AND TAPED WITH STEGO OR RAVEN TAPE. IF MATERIALS UNDERLYING THE SYNTHETIC SHEET CONTAIN SHARP, ANGULAR PARTICLES, A LAYER OF SAND APPROXIMATELY TWO INCHES THICK OR A GEOTEXTILE SHALL BE PROVIDED TO PROTECT IT FROM PUNCTURE. AN ADDITIONAL TWO-INCH THICK LAYER OF SAND MAY BE NEEDED BETWEEN THE SLAB AND THE VAPOR BARRIER TO PROMOTE PROPER CURING.

REINFORCING STEEL NOTES

- A. PROVIDE DETAILING, FABRICATION, AND INSTALLATION OF REINFORCING AND ACCESSORIES IN ACCORDANCE WITH ACI 315 AND ACI 318.
- PROVIDE NEW BILLET STEEL REINFORCING BARS IN ACCORDANCE WITH ASTM A 615, GRADE 60.
- C. PROVIDE ANCHOR BOLTS CONFORMING TO THE STANDARDS OF ASTM F1554 GR 36 UNLESS OTHERWISE NOTED.
- D. COORDINATE PLACEMENT OF CAST-IN-PLACE EMBEDMENTS AND ANCHOR BOLTS. SET ANCHOR BOLTS WITH A TEMPLATE. SECURELY ATTACH EMBEDDED ITEMS TO FORMWORK OR REINFORCING.

90-DEGREE HOOKS IN ACCORDANCE WITH ACI 318, UNLESS OTHERWISE NOTED. STAGGER SPLICES UNLESS SPECIFICALLY NOTED.

- E. MINIMUM SPLICE LENGTH SHALL BE 30 BAR DIAMETERS UNLESS NOTED OTHERWISE ON THE PLANS OR STANDARD DETAILS. PROVIDE STANDARD
- F. MAINTAIN THE FOLLOWING CONCRETE COVERAGE FOR REINFORCING STEEL UNLESS OTHERWISE NOTED:
- 1. CONCRETE CAST AGAINST EARTH: 3"
- 2. CONCRETE EXPOSED TO WEATHER:
 a. NO. 6 AND LARGER 2"
 b. NO. 5 AND SMALLER 1 1/2"
- G. DO NOT WELD OR BEND REINFORCEMENT IN THE FIELD UNLESS SPECIFICALLY SHOWN OR APPROVED BY STRUCTURAL ENGINEER.
- H. WHERE REQUIRED, PROVIDE DOWELS TO MATCH SIZE AND SPACING OF MAIN REINFORCING.
- I. PROVIDE CONTINUOUS HORIZONTAL WALL/GRADE BEAM REINFORCEMENT WITH 90-DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL BAR PLACING DETAILS.
- J. PROVIDE BAR SUPPORT ACCESSORIES IN ACCORDANCE WITH THE LATEST ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES. SUPPORT BEAM REINFORCING ON BEAM BOLSTERS SPACED NOT MORE THAN 4 FEET ON CENTER.
- K. PROVIDE BAR SUPPORTS WITH PLASTIC COATED LEGS OR HOT DIP GALVANIZING AFTER FABRICATION FOR CONCRETE EXPOSED TO VIEW. PROVIDE STAINLESS STEEL BAR SUPPORTS FOR CONCRETE TO RECEIVE A SANDBLAST FINISH.

CONCRETE NOTES

- A. PROVIDE DIFFERENT CLASSES OF CONCRETE AS SHOWN BELOW. PROVIDE BATCH MIXING, TRANSPORTATION, PLACING AND CURING OF CONCRETE IN ACCORDANCE WITH RECOMMENDATIONS OF ACI 301 AND ACI 318. USE TYPE I PORTLAND CEMENT (ASTM C150) UNLESS OTHERWISE NOTED. PROVIDE ADMIXTURES AND SPECIAL REQUIREMENTS AS SPECIFIED.
- NORMAL WEIGHT(150 PCF), F'c = 3,500 PSI CONCRETE AT 28 DAYS (MINIMUM OF 5 SACKS OF PORTLAND CONCRETE PER CUBIC YARD).
 a. ALL CONCRETE GRADE SUPPORTED SLABS AND GRADE BEAMS.
 b. FOOTINGS.
- B. UNLESS SPECIFIED BELOW, CONCRETE MUST REACH THE FOLLOWING PERCENTAGES OF ITS 28 DAY COMPRESSIVE STRENGTH (F'c) BEFORE FORMS MAY BE REMOVED:
- REFERENCE THE APPROPRIATE DISCIPLINE DRAWINGS FOR SUBSLAB PIPING, FLOOR DRAINS AND SLAB AND WALL PENETRATIONS.

EPOXY ANCHORED DOWELS AND BOLTS

- A. REFERENCE DRAWINGS FOR EPOXY ANCHORING SYSTEM.
- B. INSTALL BOLTS AND DOWELS IN ACCORDANCE WITH CURRENT ICBO REPORT FOR THE BOLT, AND RECOMMENDATIONS OF THE MANUFACTURER. FOLLOW MANUFACTURER'S INSTRUCTIONS EXPLICITLY.
- C. INSTALL BOLTS AND DOWELS PERPENDICULAR TO THE FACE OF CONCRETE, UNLESS OTHERWISE INDICATED IN THE DRAWINGS.
- D. DIAMETER OF DRILLED HOLES SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT REDIRECT HOLES ALONG LENGTH OF BOLT.
- E. DRILL HOLES IN CONTINUOUS OPERATION. BLOW DUST FROM THE HOLE USING COMPRESSED AIR.
- F. FILL ABANDONED HOLES WITH EPOXY GROUT.
- G. INSTALLATION OF EPOXY ANCHORED DOWELS AND BOLTS SHALL BE CONTINUOUSLY INSPECTED BY THE TESTING LABORATORY TO ENSURE THAT HOLES ARE OF THE PROPER DIAMETER AND LENGTH, ARE PROPERLY CLEANED, AND THAT BOLTS ARE INSTALLED CORRECTLY.
- H. UNLESS NOTED OTHERWISE EPOXY ANCHORS, REBAR, OR THREADED RODS, SHALL BE EITHER HILTI HIT HY-200 MAX ANCHORS OR SIMPSON EPOXY TIE (SET) ANCHORS. INSTALL ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS.

STRUCTURAL WOOD NOTES

- A. WOOD FRAMING:
- 1. ALL WOOD STRUCTURES HAVE BEEN DESIGNED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- 2. ALL DIMENSIONAL LUMBER SHALL BE EITHER DOUGLAS FIR OR SOUTHERN YELLOW PINE STAMPED BY A MEMBER OF THE WWPA AND KILN DRIED TO 19% MAXIMUM MOISTURE CONTENT. THE GRADES OF LUMBER TO BE USED ARE AS FOLLOWS:
- a. STUDS AND BLOCKING STUD, NO. 2 b. Joists. Rafters and Headers. No.
- b. JOISTS, RAFTERS AND HEADERS, NO. 1 c. LEDGERS, NO. 2
- d. BEAMS AND STRINGERS, NO. 1 e. COLUMNS AND POSTS, NO. 1
- 3. PRESSURE TREAT ALL WOOD PRODUCTS IN DIRECT CONTACT WITH THE FOUNDATION PER SPECIFICATION.
- 4. JOISTS, RAFTERS AND BEAMS SHALL NOT BE NOTCHED, EXCEPT WHERE SHOWN IN DETAILS. OBTAIN STRUCTURAL ENGINEER'S APPROVAL FOR ANY HOLES THROUGH OR NOTCHES IN THE TOP OF HORIZONTAL MEMBERS.
- 5. SILL PLATES SHALL BE 2x (STUD WIDTH) TREATED D.F., UNO. SILL PLATES AT ALL SHEAR WALLS SHALL BE FASTENED TO THE FOUNDATION AS SCHEDULED. SILL PLATES AT EXTERIOR WALLS SHALL BE FASTENED TO THE FOUNDATION WITH 5/8" DIAMETER x 10" LONG ANCHOR BOLTS SPACED AT 48" ON CENTER. LOCATE ANCHOR BOLTS A MAXIMUM OF 9" FROM EACH END OF PLATE. SILL PLATES AT INTERIOR WALLS SHALL BE FASTENED TO THE FOUNDATION WITH ONE 5/8" DIAMETER x 7" LONG ANCHOR BOLT AT 48" ON CENTER.
- 6. ALL HARDWARE USED IN THE WORK SHALL BE SIMPSON STRONG-TIE, KC METALS, SILVER OR OTHER HARDWARE OF EQUAL CAPACITY AND
- 7. A DOUBLE PLATE MADE OF TWO MEMBERS OF THE SAME WIDTH AS THE STUDS SHALL BE PLACED AT THE TOP OF EVERY BEARING PARTITION OR EXTERIOR WALL. SUCH DOUBLE PLATES SHALL BE LAPPED AT CORNER, AND JOINTS IN UPPER AND LOWER MEMBERS SHALL BE AT LEAST 4
- 8. EXTERIOR WALLS AND BEARING WALLS SHALL BE FRAMED, UNLESS NOTES OTHERWISE, AS FOLLOWS:
- MEMBER SIZE AND SPACING
 a. 2x6 STUDS AT 16" ON CENTER W/ BLOCKING 8'-0" O.C. MIN.

 ROOF.
- 9. ALL NAILING SHALL BE DONE IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF THE DESIGN BUILDING CODE, EXCEPT AS OTHERWISE SHOWN AND SHALL BE DONE WITH COMMON WIRE NAILS UNLESS ANOTHER TYPE OF NAIL IS ALLOWED BY THE DESIGN BUILDING CODE. (RE: NAILING SCHEDULE)
- 10. BOLTS SHALL BE UNFINISHED BOLTS CONFORMING TO ASTM A307. BOLTS HOLES THROUGH THE WOOD SHALL BE DRILLED NO MORE THAN 1/16" GREATER THAN BOLT DIAMETER.
- 11. ALL STRUCTURAL SHEATHING SHALL BE APA RATED SHEATHING. UNLESS NOTED OTHERWISE, EXTERIOR WALL CONSTRUCTION SHALL CONSIST OF 15/32" SHEATHING FULL HEIGHT. PROVIDE 8d COMMON NAILS AT 6" CENTER TO CENTER AT SUPPORT EDGES, AND AT 12" CENTER TO CENTER AT INTERMEDIATE STUDS, EXCEPT AT DESIGNATED SHEAR WALLS. PROVIDE SOLID BLOCKING AT ALL PANEL EDGES TO RECEIVE EDGE NAILING.
- 12. SOLID BLOCKING AT EDGES REQUIRED FOR ALL SHEARWALLS AND ROOF DIAPHRAMS.

R CLUE LAMINATED MEMBERS:

- ALL GLUE LAMINATED MEMBERS SHALL CONFORM TO THE APPLICABLE CURRENT REQUIREMENTS OF AITC 117 SPECIFICATION AND ALL APPROVED SUPPLEMENTS THEREOF. FABRICATION SHALL BE PERFORMED BY AN "APPROVED FABRICATOR" PER REQUIREMENTS OF GOVERNING CODE, WITH WORK AITC FACTORY INSPECTED, CERTIFIED AND GRADE STAMPED.
- 2. ALL MEMBERS SHALL MEET OR EXCEED THE FOLLOWING STRESS VALUES:
- 24F-V4 DOUGLAS FIR
- a. Fbxx (Tension zone in Tension) 2400 psi b. Fbxx (Compression zone in Tension) 1450 psi
- c. Fc perp. (TENSION FACE) 650 PSI
- d. Fc perp. (COMPRESSION FACE) 650 PSI
- e. Fvxx (HORIZONTAL SHEAR) 240 PSI f. E (MODULUS OF ELASTICITY) 1800000 PSI
- 3. LAMINATED MEMBERS SHALL BE SIZED TO DIMENSIONS SHOWN ON DRAWINGS WITH BEAMS CUT TO DEPTHS WITHIN AN APPROVED TOLERANCE ±
- 1/8". ENDS ARE TO BE SEALED AND INDIVIDUALLY WRAPPED.

 4. ALL LAMINATED BEAMS SHALL BE DELIVERED WITH MOISTURE CONTENT WHICH IN IS IN COMPLIANCE WITH THE AITC SPECIFICATIONS
- CONCERNING THAT SPECIES.
- 5. ALL LAMINATED BEAMS SHALL BE CAPABLE OF DEVELOPING THE FULL SPECIFIED DESIGN STRESSES, I.D. BLEMISHES, IMPERFECTIONS, SPLITS, ETC., SHALL NOT EXCEED THAT WHICH HAS BEEN ACCOUNTED FOR IN ESTABLISHING THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION ALLOWABLE DESIGN STRESSES.

TYPICAL LUMBER NAILING SCHEDULE

6. ANY CLUE LAMINATED MEMBER WHICH DOES NOT MEET THE ABOVE DESIGN REQUIREMENTS SHALL BE REJECTED. USE OF THE FEELER GAGE TEST METHOD OF MEASURING DEPTHS OF SPLITS, ETC., WILL NOT BE PERMITTED FOR THE PURPOSE OF DETERMINING MEMBERS ACCEPTABILITY, BY MAY BE UTILIZED FOR REJECTION.

NAILI	NG SHOWN IS TYPICAL EXCEPT AS NOTED ON PLANS. USE COMMON	NAILS.
1.	JOIST TO SILL OR GIRDER, TOENAILS	3-8d
2.	BRIDGING TO JOIST, TOE NAIL EACH END	2-8d
3.	1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
4.	WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
5.	2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
6.	SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d AT 16" O.C.
7.	TOP PLATE TO STUD, END NAIL	2-16d
8.	STUD TO SOLE PLATE	2-16d END NAILS OR 4-8d TOE NAILS
9.	DOUBLE STUDS, FACE NAIL	16d AT 12" STAGGERED
10.	DOUBLED TOP PLATES, FACE NAIL	16d AT 16" O.C.
11.	TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
12.	CONTINUOUS HEADER, TWO PIECES	16d AT 16" O.C. ALONG EA. EDGE
13.	CEILING JOISTS TO PLATE, TOE NAIL	3-8d
14.	CONTINUOUS HEADER TO STUD, TOE NAIL	4-8d
15.	CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3–16d
16.	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3–16d
17.	RAFTER TO PLATE, TOENAIL	3-8d
18.	1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
19.	1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	2-8d
20.	WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL	3-8d
21.	BUILT-UP CORNER STUDS	16d AT 24" O.C.
22.	BUILT-UP GIRDER AND BEAMS	20d AT 32" O.C. AT TOP & BOTTOM 2-20d AT ENDS AND AT EA. SPLICE
23.	2" PLANKS	2-16d AT EACH BEARING

SPECIAL INSPECTIONS NOTES

- A. THE OWNER WILL EMPLOY THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION FOR THE ITEMS IN THE SPECIAL INSPECTION TABLE BELOW IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2006 EDITION. NOT APPLICABLE TO SLABS ON GROUND OR SITE CONCRETE FULLY SUPPORTED ON EARTH.
- B. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
- C. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
- 1. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAIVE ANY OF THE
- 2. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL OF RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE
- 3. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.
- D. OBSERVATION BY STRUCTURAL ENGINEER OF RECORD IS NOT REQUIRED. IF STRUCTURAL OBSERVATION IS REQUIRED BY THE BUILDING AUTHORITY IT SHALL BE AS DIRECTED BY THE OWNER TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE.
- E. THE FABRICATION OF TRUSSES AND OTHER ASSEMBLIES CONSTRUCTED USING WOOD AND METAL MEMBERS, OR USING LIGHT METAL PLATE CONNECTORS, SHALL BE CONTINUOUSLY INSPECTED BY A QUALIFIED INSPECTOR APPROVED BY THE ENFORCEMENT AGENCY AND PAID FOR BY THE CONTRACTOR. THE INSPECTOR SHALL FURNISH THE ARCHITECT, STRUCTURAL ENGINEER AND THE ENFORCEMENT AGENCY WITH A REPORT THAT THE LUMBER SPECIES, GRADES AND MOISTURE CONTENT, TYPE OF GLUE, TEMPERATURE AND GLUING PROCESS, TYPE OF METAL MEMBERS AND METAL CONNECTORS, AND THE WORKMANSHIP, CONFORM IN EVERY MATERIAL RESPECT WITH THE DULY APPROVED PLANS AND SPECIFICATIONS. EACH INSPECTED TRUSS SHALL BE STAMPED BY THE INSPECTOR WITH AN IDENTIFYING MARK.
- F. INSPECTION OF FABRICATORS IS NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED BY THE BUILDING OFFICIAL TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS TO THE BUILDING OFFICIAL AND TO THE ENGINEER OF RECORD.
- G. SPECIAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH THE TABLE BELOW:

SPECIAL INSPECTION	FREQ.	REFERENCED STANDARD					
SOILS:							
I. PRIOR TO THE PLACEMENT OF PREPARED FILLS, VERIFY THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE FOUNDATION NOTES AND GEOTECHNICAL REPORT.							
2. FILL PLACEMENT 12" THICK OR GREATER — VERIFY THAT THE MATERIAL BEING USED AND MAXIMUM LIFT THICKNESS COMPLY WITH THE FOUNDATION NOTES AND GEOTECHNICAL REPORT.	CONTINUOUS	GEOTECHNICAL ENGINEERING REPORT; IBC 1704.7					
3. VERIFY, AT THE FREQUENCY SPECIFIED IN THE FOUNDAITON NOTES AND GEOTECHNICAL REPORT, THAT THE IN-PLACE DRY DENSITY OF COMPACTEDFILL COMPLIES WITH THE FOUNDATION NOTES AND GEOTECHNICAL REPORT.	ES AND GEOTECHNICAL REPORT, THAT THE IN-PLACE DRY SITY OF COMPACTEDFILL COMPLIES WITH THE FOUNDATION						
CONCRETE:							
I. INSPECTION OF REINFORCING STEEL, SIZE AND PLACEMENT	PERIODIC	ACI 318: 3.5, 7.1-7.7					
2. INSPECT BOLTS TO BE INSTALLED IN CONCRETE, PRIOR TO AND DURING PLACEMENT OF CONCRETE.	PERIODIC	IBC 1912.5					
3. VERIFYING USE OF REQUIRED DESIGN MIX	PERIODIC	ACI 318: Ch. 4, 5.2-5.4					
4. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP, TEMP. AND AIR CONTENT TESTS.	CONTINUOUS	ASTM C 172; ASTM C 31; ACI 318: 5.6, 5.8					
5. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	CONTINUOUS	ACI 318: 5.9, 5.10					
ADHESIVE ANCHORS/REINF.:							
I. DURING PLACEMENT OF ADHESIVE ANCHORS OR REINFORCEMENT EMBEDDED WITH ADHESIVE (AS SPECIFIED ON THE CONSTRUCTION DOCUMENTS) IN MASONRY AND CONCRETE:	CONTINUOUS	MANUFACTURERS					
 G. SIZE AND EMBEDMENT OF ANCHORS/REINF. b. ANCHORS/REINF. INSTALLED PER MANUFACTURERS RECOMMENDATIONS. 	CONTINUOUS	MANUFACTURERS INSTALLATION INSTRUCTIONS					
STRUCTURAL WOOD:							
I. INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC—FORCE—RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS AND HOLD—DOWNS, WHERE THE FASTENERS SPACING OF THE SHEATHING IS MORE THAN 4" O.C.	PERIODIC	IBC 1707.3					

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PROJECT INFORMATION:



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PROJECT NO.: 2015-1674

DRAWN BY: SMM

CHECKED BY: JRD

ISSUE: DATE:

PERMIT SET 03-18-16

SION: DATE:

KOKOMO, IN

PROJECT LOCATION:

SHEET NUMBER / TITLE S1.01

WOOD SHEARWALL SCHEDULE														
MARK	PANEL GRADE	THICKNESS (in) MIN. NOMINAL PANEL	IN FRAMING (in) MIN. NAIL PENETRATION	NAIL SIZE	NAIL SPACING				HOLDOWNS AT THE ENDS					
					PANEL EDGES	INTERMEDIATE FRAMING	ANCHOR BOLTS SILL PLATE	END POSTS	TYPE	ANCHOR BOLT	ANCHOR BOLT VALUES (inches)			NOTES
											EMBED	EDGE DISTANCE	END DISTANCE	
SW1	SIMPSON STRONG-WALL® SB SHEARWALL MODEL SWSB24x20	N/A	N/A	N/A	N/A	N/A	N/A	N/A	SIMPSON PROVIDED PRE-ATTACHED TO SWSB24x20	1" DIA.	20	9	9	see extra Reinf detail
SW2	C-C, C-D APA RATED SHEATHING ON INTERIOR AND EXTERIOR FACE OF WALL	15/32	1–1/2	10d	4	12	3x SILL 5/8" DIA. BOLT @ 16" OC	6x6	SIMPSON HD14-SDS2.5 OR EQUIVALENT	1" DIA.	20	9	9	see extra Reinf detail
SW3	C-C, C-D APA RATED SHEATHING ON EXTERIOR FACE OF WALL	15/32	1–1/2	10d	4	12	2x SILL 5/8" DIA. BOLT @ 24" OC	(3) 2x6	SIMPSON HDU8-SDS2.5 OR EQUIVALENT	7/8" DIA.	15	2 3/4	7	
SW4	C-C, C-D APA RATED SHEATHING ON EXTERIOR FACE OF WALL	15/32	1-1/2	10d	6	12	2x SILL 5/8" DIA. BOLT @ 48" OC	(3) 2x6	SIMPSON HDU2-SDS2.5 OR EQUIVALENT	5/8" DIA.	10	2 3/4	7	

SHEARWALL SCHEDULE NOTES:

- 1. REFER TO 2/S2.01 FOR TYPICAL SHEARWALL ELEVATION.
- ALL PANEL ÉDGES SHALL BE BACKED WITH 2 INCH NOMINAL OR WIDER FRAMING.
 PANELS INSTALLED ON 2X6 STUDS SPACED 16 INCHES ON CENTER.

REINFORCING

TOP OF FOOTING = 94'-0"

TOP OF FOOTING = 94'-0"

(4) #5 CONTINUOUS

(5) #5 CONTINUOUS

- 4. WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 6 INCHES ON CENTER ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3 INCHES NOMINAL OR THICKER AND NAILS ON EACH SIDE SHALL BE STAGGERED.
- 5. WHERE 10d NAILS ARE SPACED 3 INCHES OR LESS FRAMING AT ADJOINING PANEL EDGES SHALL BE 3 INCH NOMINAL OR WIDER AND STAGGERED.
- 6. WHERE SILL PLATE ANCHOR BOLTS ARE MISSING OR MISLOCATED, USE HILTI HIT—HY100 OR EQUIVALENT WITH
 5 INCHES MIN. EMBEDMENT WITH THREADED ROD OF THE SAME SIZE AND SPACING AS INDICATED.
 7. IN ADDITION TO THE ON CENTER SPACING OF ANCHOR BOLTS, THERE SHALL BE AT LEAST (2) ANCHOR BOLTS
- PER PIECE OF SILL PLATE WITH A MINIMUM OF (1) ANCHOR BOLT LOCATED NOT GREATER THAN 7 INCHES, AND LESS THAN 4-1/2 INCHES OF EACH PIECE END.
- 8. MINIMUM CONCRETE SIDE EDGE DISTANCE FOR BOTH SILL PLATE AND HOLDDOWN ANCHOR BOLTS IS 23/4 INCHES AND MINIMUM CONCRETE END EDGE DISTANCE FOR HOLDDOWN ANCHOR BOLTS IS 7 INCHES, UNO.
- HOLDDOWNS SHALL BE INSTALLED PER MANUFACTURER INSTALLATION REQUIREMENTS.
 HOLES ARE NOT ALLOWED IN SHEARWALLS, UNLESS APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION. IF A HOLE IS APPROVED, IT IS TO BE ENTIRELY WITHIN ONE SHEET. PROVIDE BLOCKING AND
- EDGE NAILING AROUND OPENING.

 11. MIN. WIDTH OF SHEATHING SHALL BE 2 FT.

FOOTING SCHEDULE

12. NAILS SHALL BE COMMON NAILS.

FOOTING

HEIGHT

WIDTH

36"

PLAN NOTES

В

6'-10"

30'-8"

TOP OF FOOTING = 94'-0"-

17**'**–6**"**

FIN FLR EL = 100'-0"
4" CONC. SLAB PLACED OVER
10 MIL VAPOR RETARDER OVER

4" OF CRUSHED STONE, REINFORCE SLAB WITH #3 @ 16" O.C. EACH WAY.

- 1. INDICATES 6" WOOD SHEAR WALLS.
- 2. REFER TO SHEET S1.01 FOR GENERAL NOTES.
- REFER TO PLUMBING DRAWINGS FOR ALL FLOOR DRAIN, TRENCH DRAIN AND MOP SINK SIZES AND LOCATIONS.

 1. FLOOR DRAIN GRATES SHALL BE FLUSH WITH FINISHED FLOOR.
- 5. SLOPE FLOOR 1/16" PER FOOT MIN. AT ALL FLOOR DRAINS AS SHOWN & NOT TO EXCEED 1/4" PER FT.
- 6. ALL SIMPSON HOLD DOWNS TO BE STAINLESS STEEL OR Z-MAX G185 HOT DIPPED GALVANIZED COATING.
 7. ALL SILL PLATE ANCHOR BOLTS TO BE STAINLESS STEEL OR G185 HOT DIPPED GALVANIZED.
- 8. ALL STEEL IN CONTACT WITH TREATED WOOD TO BE STAINLESS STEEL OR G185 HOT DIPPED GALVANIZED.
- REFER TO GENERAL NOTES FOR TYPICAL WALL FRAMING AND ATTACHMENT.
 REFER TO GENERAL NOTES FOR TYPICAL SILL PLATE & ANCHOR BOLT REQUIREMENTS.
- 11. REFER TO 02/S3.01 FOR TYPICAL GRADE BEAM CONSTRUCTION JOINT DETAIL.

11'-0"

INSIDE CORNER

SHEARWALL SHEATHING THIS FACE ONLY

5'-4½"

12'-4"

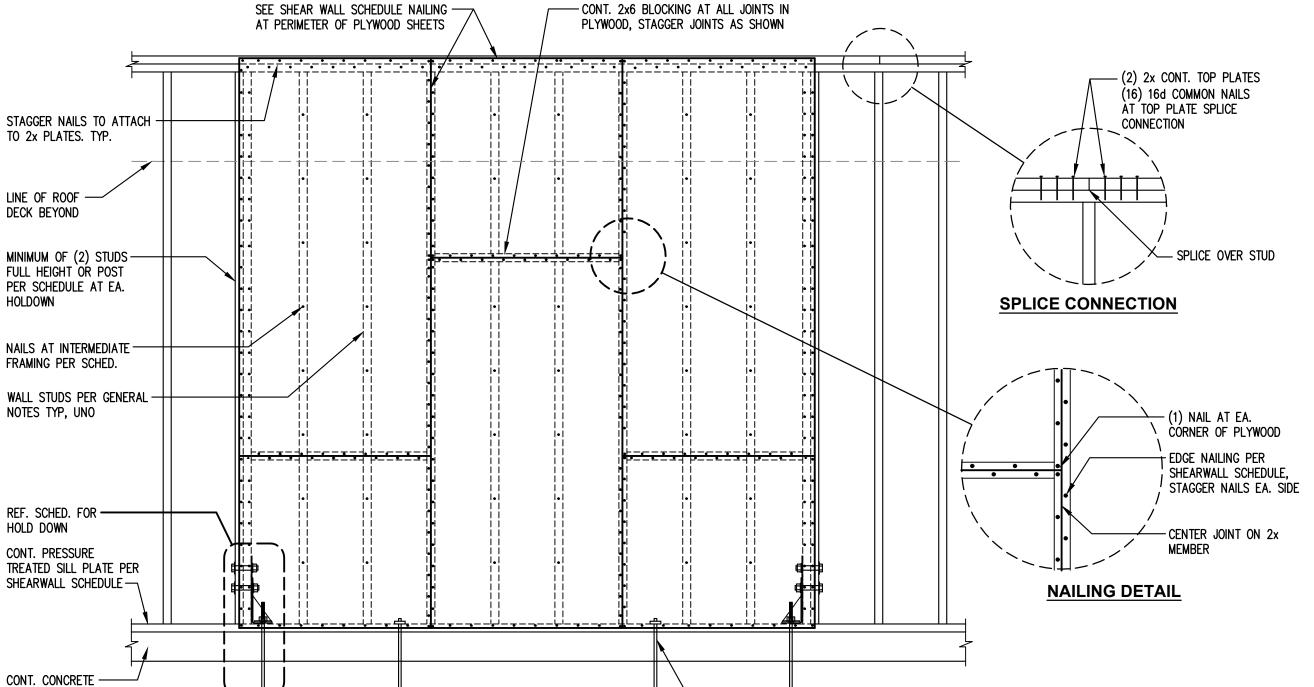
4'-1"

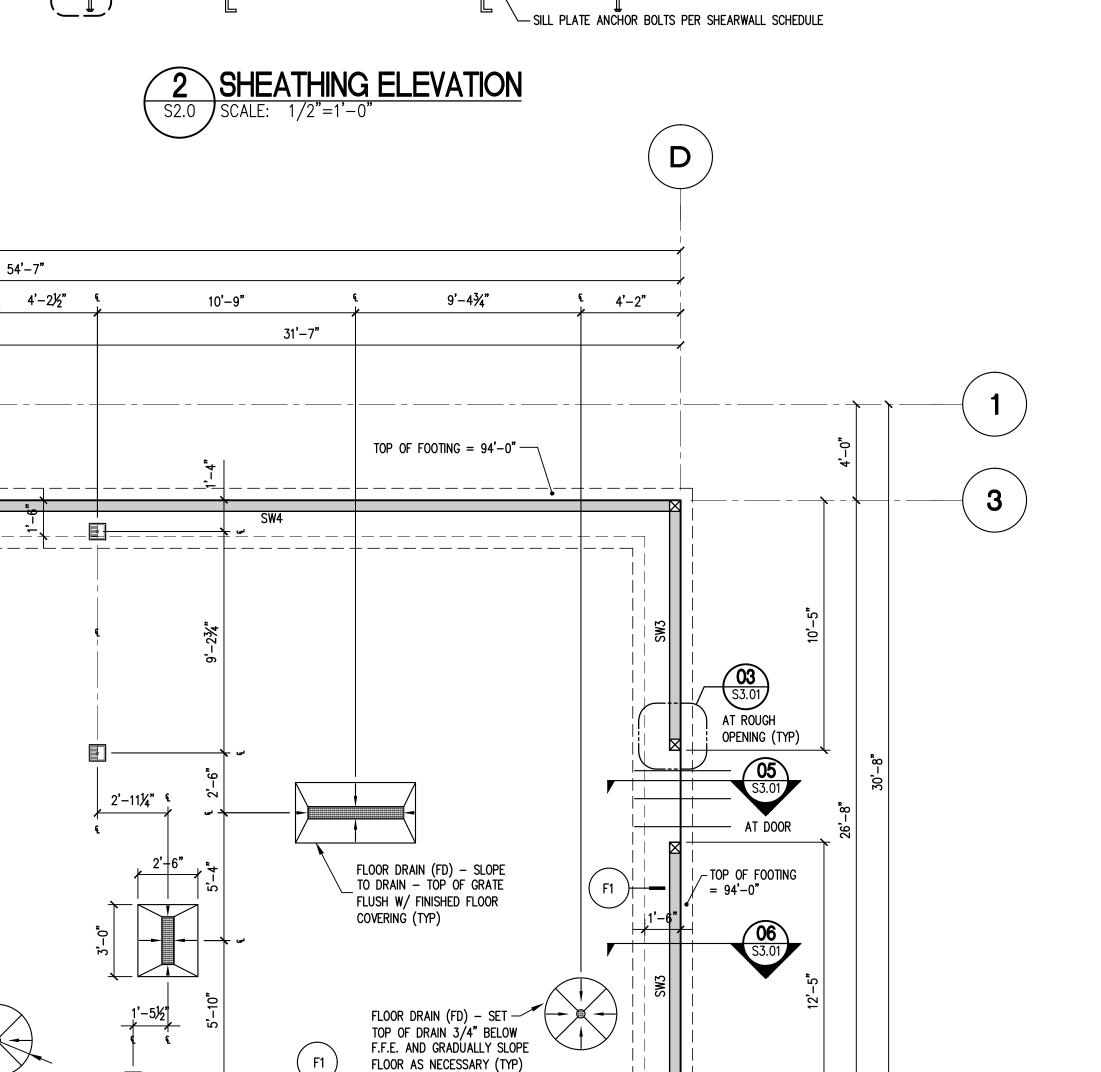
AT DOOR

2'-9¼" 5 3'-0½" 5

12. REFER TO 03/S3.01 AND 04/S3.01 FOR TYPICAL HOLD DOWN DETAILS.

13. ALL ANCHOR BOLTS TO BE F1554 GR 55 HEAVY HEX HEAD BOLTS.







83'-1"

5'-0**"**

83'-1"

5'-3**¾"**

CURBING RE: ARCH.

8'-3¼"

 \longrightarrow TOP OF FOOTING = 94'-0"

52'-5"

CONSULTANT:

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ENT:



GEAUX SONIC LLC 1012 SOUTH COOPER STREET GREENTOWN, IN 46901

PROJECT INFORMATION:

E. MARKLAND AVENUE
KOKOMO, IN 46901

EAL:

JECT NO.:	2015-1674	
WN BY:	SMM	
CKED BY:	JRD	
JE:		DATE:
RMIT SET		03-18-16

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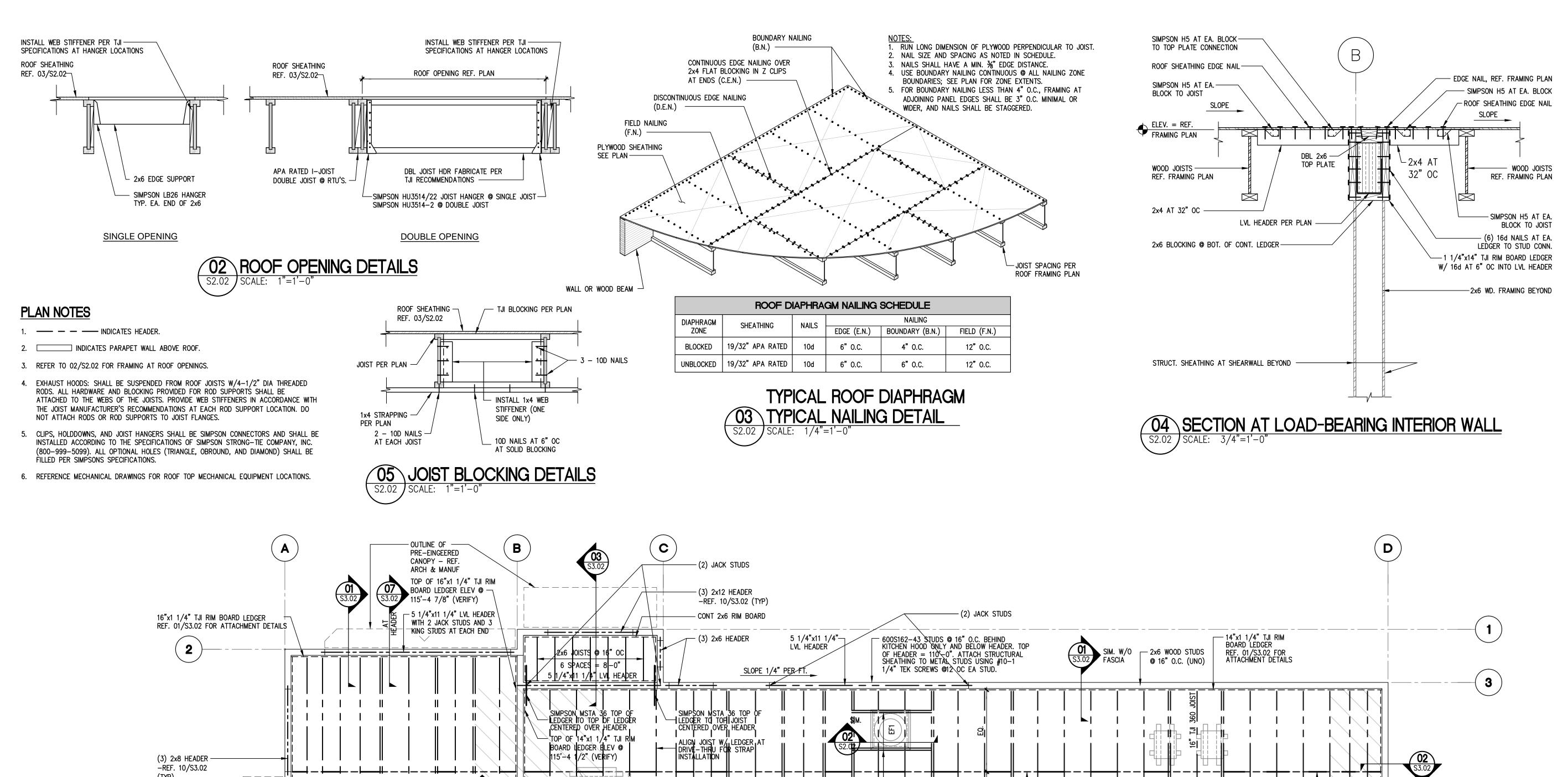
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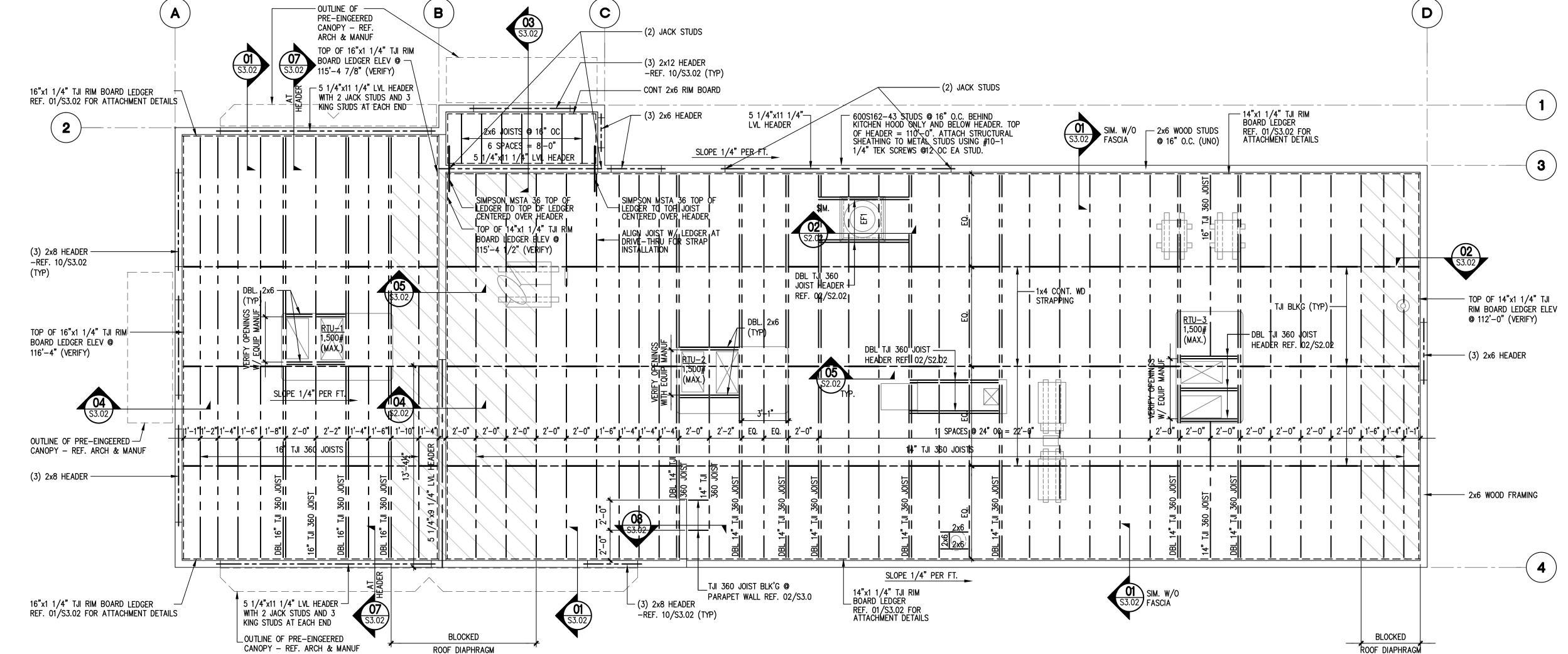
KOKOMO, IN

SHEET NUMBER / TITLE

\$2.01

FOUNDATION PLAN





1 ROOF FRAMING PLAN
S2.02 SCALE: 1/4"=1'-0"

CONSULTANT:

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GEAUX SONIC LLC 1012 SOUTH COOPER STREET GREENTOWN, IN 46901

PROJECT INFORMATION:

C DRIVE-IN RESTAURANT
E. MARKLAND AVENUE
KOKOMO, IN 46901

-

PROJECT NO.: 2015-1674

DRAWN BY: SMM

CHECKED BY: JRD

:: DATE:
#IT SET 03-18-16

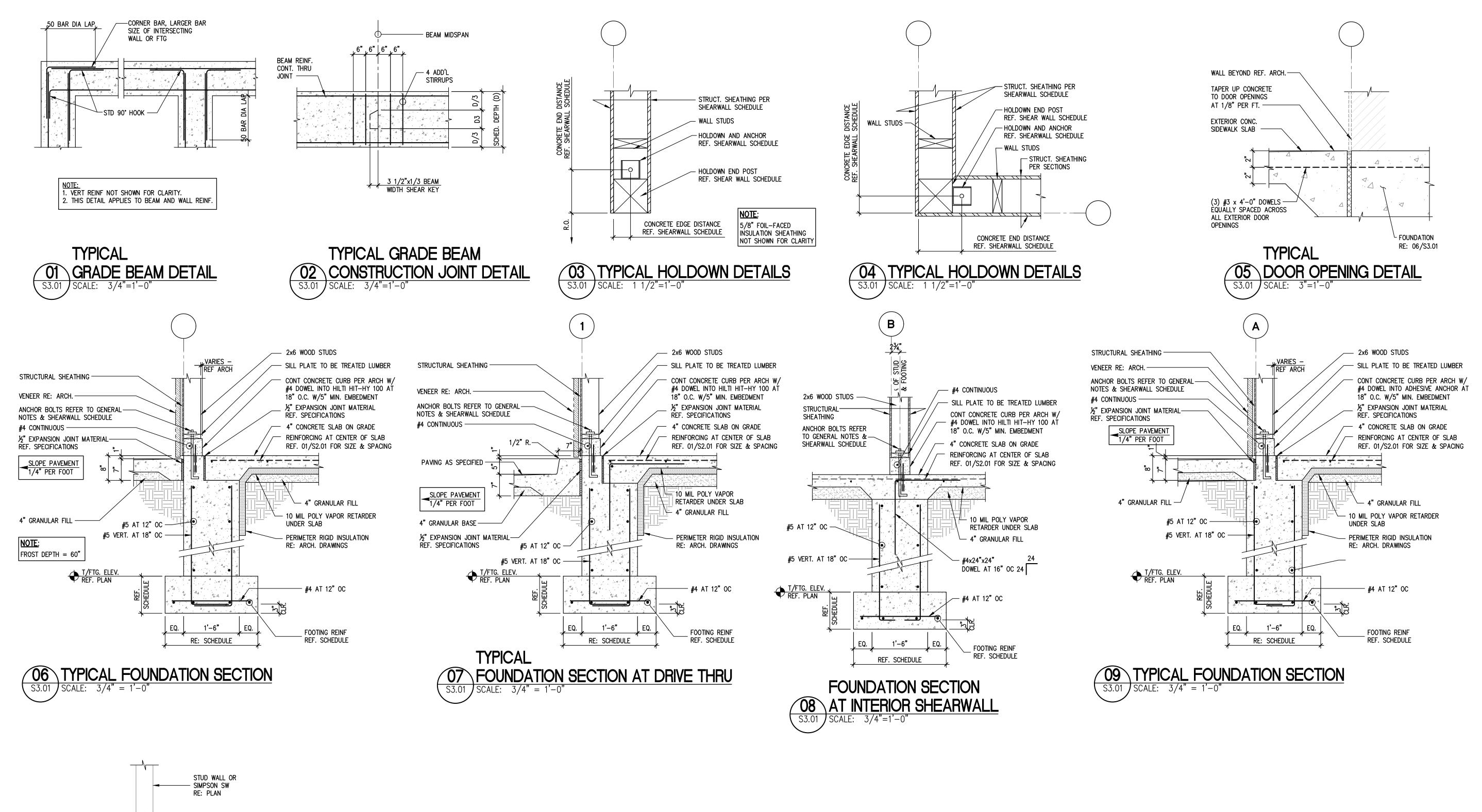
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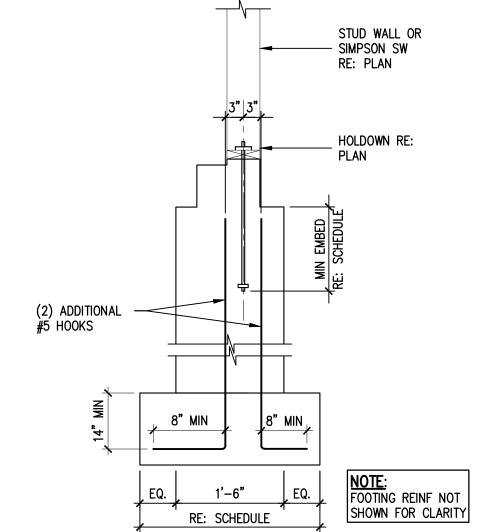
PROJECT LOCATION:

KOKOMO, IN

SHEET NUMBER / TITLE

\$\begin{align*}
\$2.02 \\
ROOF FRAMING PLAN
\end{align*}





EXTRA REINF DETAIL S3.01 SCALE: 3/4" = 1'-0"

PROJECT NO.: 2015-1674 DRAWN BY:

CONSULTANT:

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Sanger, Texas 76266

Project #12106

B. J. ISBELL, P.E.

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PROJECT INFORMATION:

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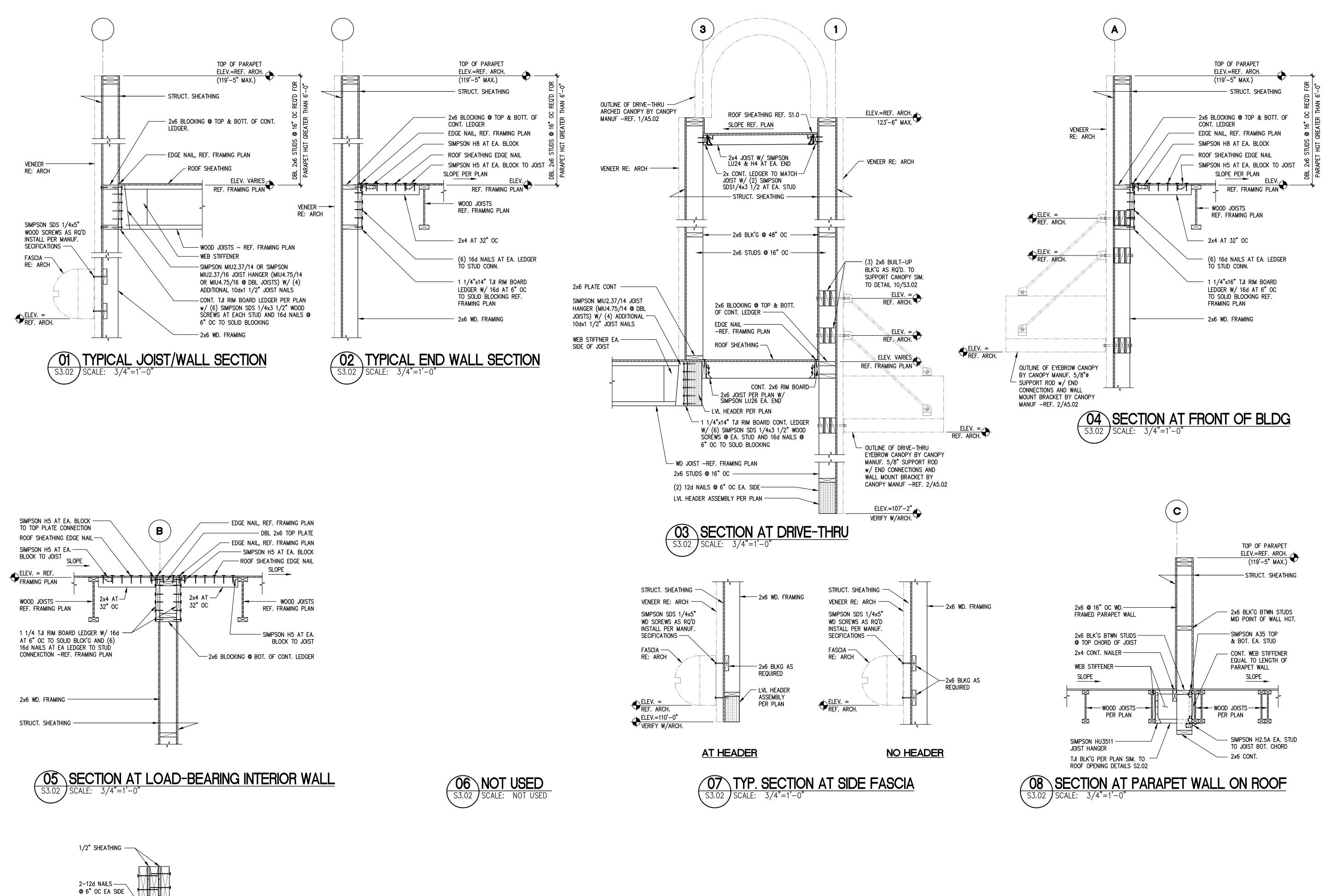
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DATE:

PROJECT LOCATION:

SHEET NUMBER / TITLE S3.01 FOUNDATION & FRAMING DETAILS

KOKOMO, IN



REF. 01/S2.02 FOR DEPTH OF HEADER

MEMBERS

TYP. 2x BUILT-UP

09 HEADER DETAIL

CONSULTANT:

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ENT:



GEAUX SONIC LLC 1012 SOUTH COOPER STREET GREENTOWN, IN 46901

PROJECT INFORMATION:

SONIC DRIVE-IN RESTAURANT 2321 E. MARKLAND AVENUE KOKOMO, IN 46901

PROJECT NO.: 2015-1674

DRAWN BY: SMM

DRAWN BY: SMM

CHECKED BY: JRD

ISSUE: DATE:

DEPMIT SET 03 19 16

PERMIT SET 03-18-16

REVISION: DATE:

PROJECT LOCATION:

KOKOMO, IN

SHEET NUMBER / TITLE

\$\begin{align*}
\$3.02 \\
\text{ROOF FRAMING DETAILS}
\end{align*}

GENERAL NOTES:

- 1. ALL DIMENSIONS TO, OF, AND IN EXISTING STRUCTURES SHALL BE VERIFIED IN FIELD BY CONTRACTOR WITH ALL DISCREPANCIES REPORTED TO THE ENGINEER.
- 2. DO NOT CHANGE THE SIZE NOR SPACING OF STRUCTURAL ELEMENTS WITHOUT THE APPROVAL OF THE ENGINEER.
- 3. THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.
- 4. THE DESIGN IS BASED ON THE INDIANA BUILDING CODE, 2008 EDITION.
- 5. CONTRACTOR SHALL DETERMINE EXACT LOCATION OF EXISTING UTILITIES BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE UNDERGROUND UTILITIES.
- 6. INCORRECTLY FABRICATED, DAMAGED, OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE OWNER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER 'S APPROVAL.

CANOPY NOTES:

- 1. CANOPIES ARE TO REMAIN LEVEL WITH BUILDING.
- 2. ALL WELDING ON ARCHED CANOPY IS TO BE PERFORMED BY THE CANOPY MANUFACTURER
- WITHIN THEIR FABRICATION FACILITY. 3. DO NOT SLOPE CANOPIES WITH GRADE.
- 4. SEE CIVIL OR SITE PLANS FOR LOCATION DIMENSIONS.
- 5. SEE ARCHITECTURAL SHEET A7.01 FOR CANOPY ATTACHMENT DETAILS.

FOUNDATION AND SOIL PREPARATION NOTES:

- 1. THE FOUNDATION DESIGN IS BASED ON ALLOWABLE BEARING CAPACITY OF 1,500 PSF AS REFERENCED ON SHEET S1.01.
- 2. ALL PIERS SHALL BE EXCAVATED AND CONCRETE SHALL BE PLACED THE SAME DAY OR THE PIER IS TO BE RE-EXCAVATED PER THE INSTRUCTIONS OF THIS OFFICE.
- 3. BEAR ALL PIERS ON NATIVE UNDISTURBED SOIL. SOIL BEARING SURFACES WHICH ARE ALLOWED TO BECOME SATURATED, FROZEN OR DISTURBED SHALL BE REWORKED TO THE SATISFACTION OF OWNER'S REPRESENTATIVE.

DRILLED SHAFT (PIER) NOTES:

- 1. CONCRETE FOR DRILLED SHAFTS SHALL HAVE A 28 DAY DESIGN COMPRESSIVE STRENGTH OF 3,000 PSI, A MINIMUM OF 470 POUNDS OF PORTLAND CEMENT PER CUBIC YARD, MINIMUM 1% TOTAL AIR CONTENT (ENTRAPPED), MID OR HIGH RANGE WATER REDUCING AGENT AND A 6-7" INCH SLUMP.
- 2. THE BOTTOM OF THE SHAFT SHALL BE CLEAN AND FREE OF LOOSE SOIL PRIOR TO PLACING CONCRETE.
- 3. CONCRETE FOR DRILLED SHAFTS SHALL HAVE A 28 DAY DESIGN COMPRESSIVE STRENGTH OF 3,000 PSI, A MINIMUM OF 470 POUNDS OF PORTLAND CEMENT PER CUBIC YARD, MINIMUM 4-6% TOTAL AIR CONTENT (ENTRAINED), MID OR HIGH RANGE WATER REDUCING AGENT AND A 6-7" INCH SLUMP. AIR ENTRAINING AGENT OR FLYASH SHALL NOT BE USED WITHOUT THE APPROVAL OF THE ENGINEER PRIOR TO BIDDING.
- 4. NO SPLICING OF THE DRILLED SHAFT STEEL SHALL BE ALLOWED.

CONCRETE NOTES:

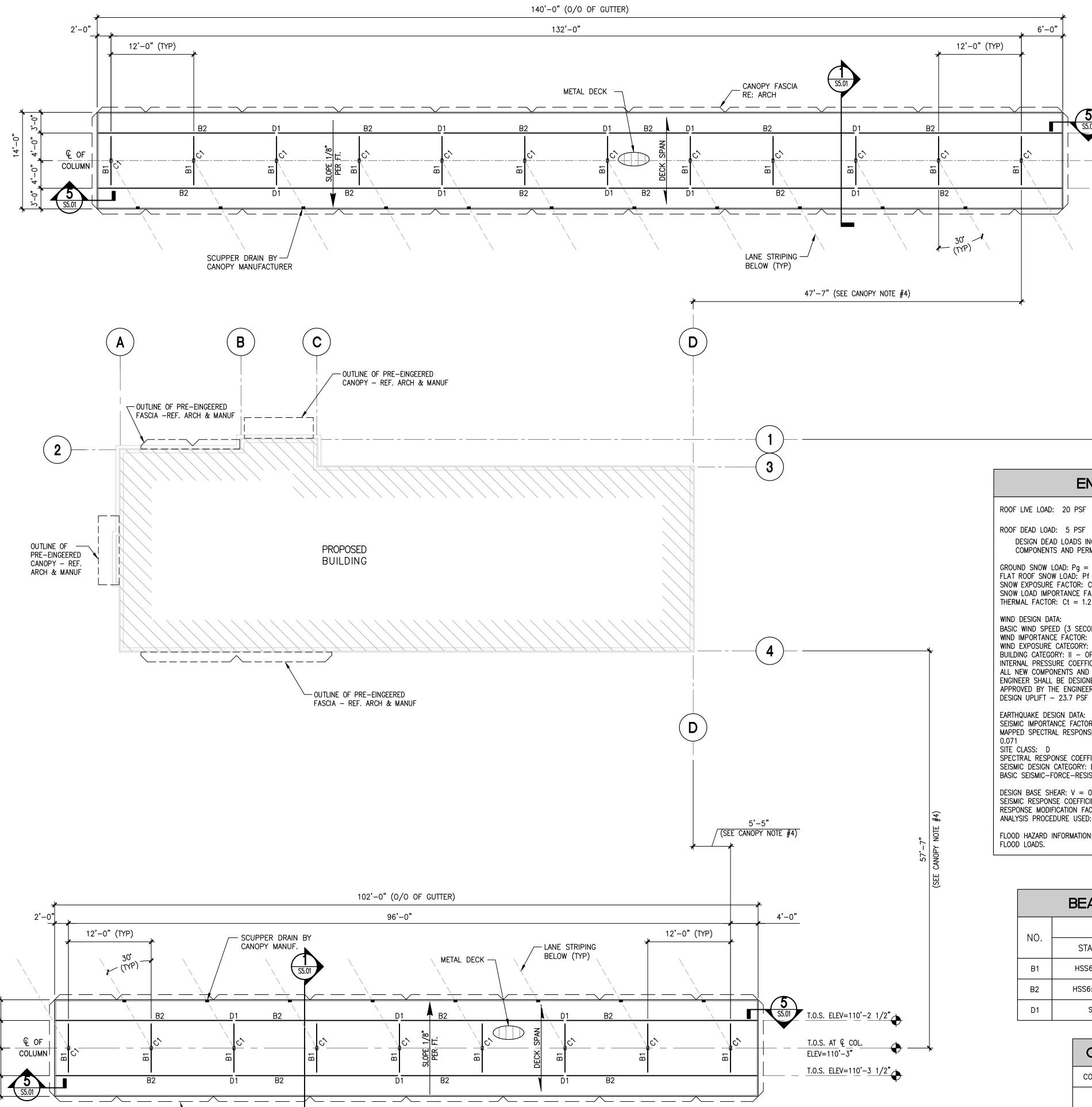
- 1. CONCRETE FOR PATIO SLAB SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI, A MINIMUM OF 5 SACKS OF PORTLAND CEMENT PER CUBIC YARD, 5-7% AIR CONTENT USING AIR ENTRAINING AGENT AS REQUIRED, AND A 5-6" SLUMP.
- 2. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE STANDARDS "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318-11) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301-10).
- 3. REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO A.S.T.M. A-615 GRADE 60.
- 4. REINFORCING BAR, BAR SUPPORTS, AND SPACERS SHALL BE DETAILED AND PROVIDED IN ACCORDANCE WITH A.C.I. DETAILING MANUAL. CHAIRS SHALL NOT BE PLACED FURTHER THAN 4 FEET APART.
- 5. EPOXY ANCHORS, REBAR, OR THREADED RODS SHALL BE EITHER HILTI HIT HY-200 MAX ANCHORS OR SIMPSON EPOXY-TIE (ET) ANCHORS. INSTALL ACCORDING TO THE

MANUFACTURER'S RECOMMENDATIONS. THIS INCLUDES CLEANING THE HOLE WITH AIR.

STEEL NOTES:

- 1. STRUCTURAL STEEL FABRICATION AND ERECTION SHALL CONFORM TO THE A.I.S.C. MANUAL OF
- 2. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY.
- 3. ANY CONNECTIONS WITHOUT WELD SYMBOLS SHALL BE AT A MINIMUM WELDED ALL AROUND WITH THE MINIMUM FILLET OR BUTT WELD SIZE.
- 4. STRUCTURAL STEEL ANGLES, PLATES, ETC. SHALL CONFORM TO A.S.T.M. A36 REQUIREMENTS (36 KSI). STRUCTURAL TUBING AND PIPES SHALL CONFORM TO THE A.S.T.M. A500 GRADE B REQUIREMENTS (46 KSI).
- 5. DO NOT PLACE HOLES THROUGH STRUCTURAL STEEL MEMBERS EXCEPT AS SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.
- 6. ALL BOLTED CONNECTIONS ARE TO BE A325 TYPE N BOLTS IN STANDARD HOLES UNLESS

NOTED OTHERWISE AND SHALL BE MADE USING THE TURN OF THE NUT METHOD.



ENGINEERING DATA

T.O.S. ELEV=110'-2 1/2"

T.O.S. ELEV=110'-3 1/2"

ROOF LIVE LOAD: 20 PSF

ROOF DEAD LOAD: 5 PSF

DESIGN DEAD LOADS INCLUDE THE WEIGHT OF STRUCTURAL COMPONENTS AND PERMANENT FIXTURES.

GROUND SNOW LOAD: Pg = 20 PSF FLAT ROOF SNOW LOAD: Pf = 20 PSF SNOW EXPOSURE FACTOR: Ce = 1.0SNOW LOAD IMPORTANCE FACTOR: I = 1.0THERMAL FACTOR: Ct = 1.2

WIND DESIGN DATA: BASIC WIND SPEED (3 SECOND GUST): 90 MPH WIND IMPORTANCE FACTOR: I = 1.0WIND EXPOSURE CATEGORY: C BUILDING CATEGORY: II — OPEN BUILDING INTERNAL PRESSURE COEFFICIENTS: 0 ALL NEW COMPONENTS AND CLADDING NOT DESIGNED BY THE ENGINEER SHALL BE DESIGNED FOR 25 PSF UNLESS OTHERWISE APPROVED BY THE ENGINEER

EARTHQUAKE DESIGN DATA: SEISMIC IMPORTANCE FACTOR, I: 1.0 MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss = 0.152 S1 =

SPECTRAL RESPONSE COEFFICIENTS - Sds = 0.162, Sd1 = 0.113 SEISMIC DESIGN CATEGORY: B BASIC SEISMIC-FORCE-RESISTING SYSTEM: INVERTED PENDULUM TYPE STRUCTURE DESIGN BASE SHEAR: V = 0.78 kips

SEISMIC RESPONSE COEFFICIENT: Cs = 0.086 RESPONSE MODIFICATION FACTOR: R = 2.0ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE METHOD FLOOD HAZARD INFORMATION: THIS BUILDING IS NOT DESIGNED FOR

BEAM SCHEDULE BEAM SIZE NO. STANDARD ALTERNATE HSS6x4x1/4" C8x11.5 B2 HSS6x4x3/16" Z8x2.5x12GA SEE CONNECTION DETAIL 3/S5.01

COLUMN SCHEDULE								
COLUMN NUMBER	COLUMN SIZE							
C1	HSS6x4x1/4"							

VERIFY ALL DIMENSIONS WITH **CANOPY PLAN** ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS

CANOPY FASCIA RE: ARCH

CONSULTANT:

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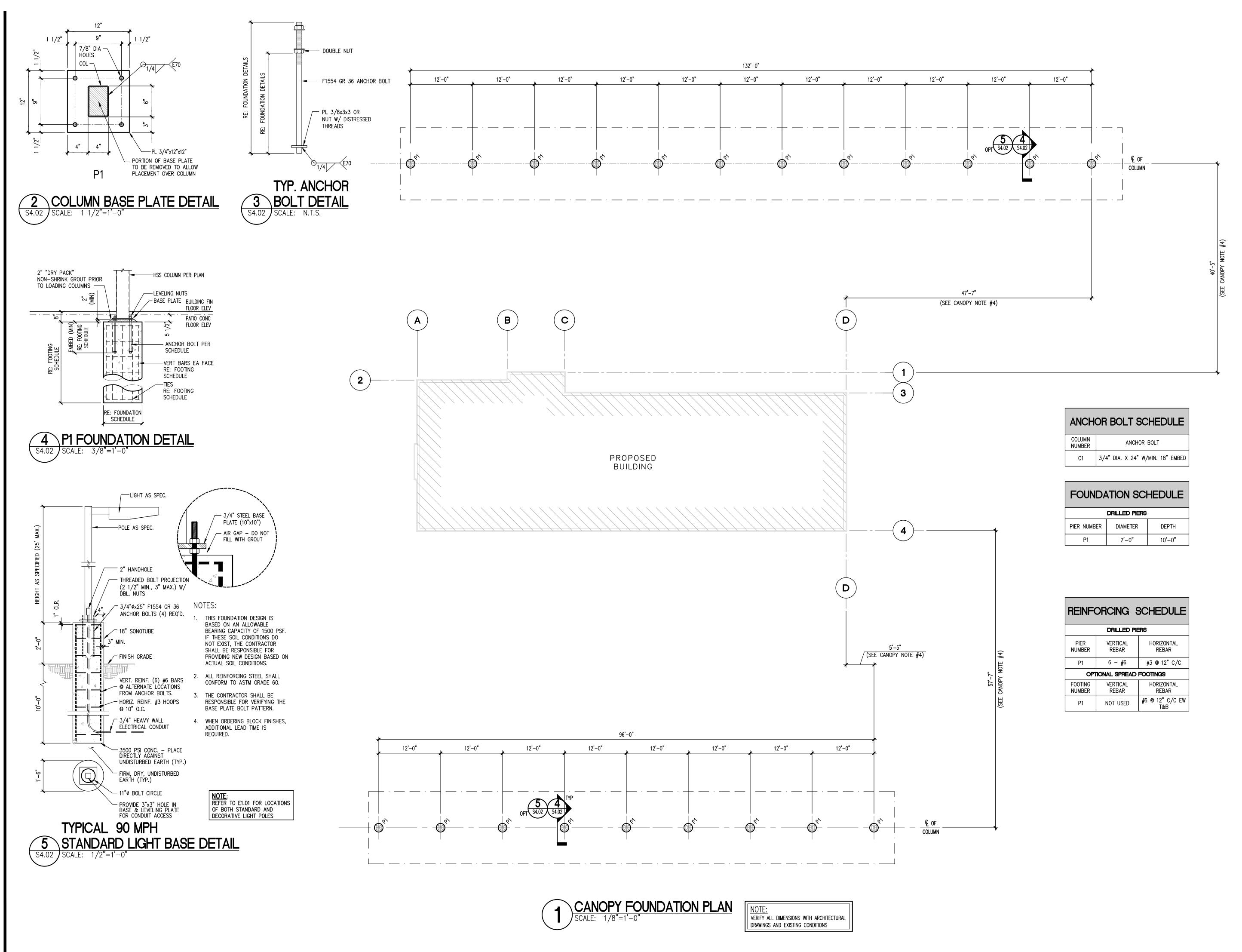
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PROJECT NO.: 2015-1674 DRAWN BY: SMM

DATE: PERMIT SET 03-18-16 DATE:

PROJECT LOCATION: KOKOMO, IN

SHEET NUMBER / TITLE **CANOPY FRAMING PLAN**



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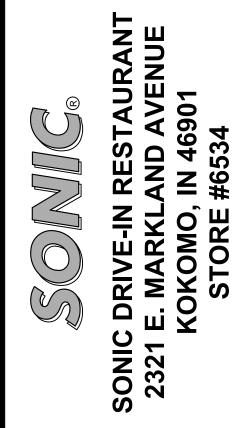
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CLIENT



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PROJECT INFORMATION:



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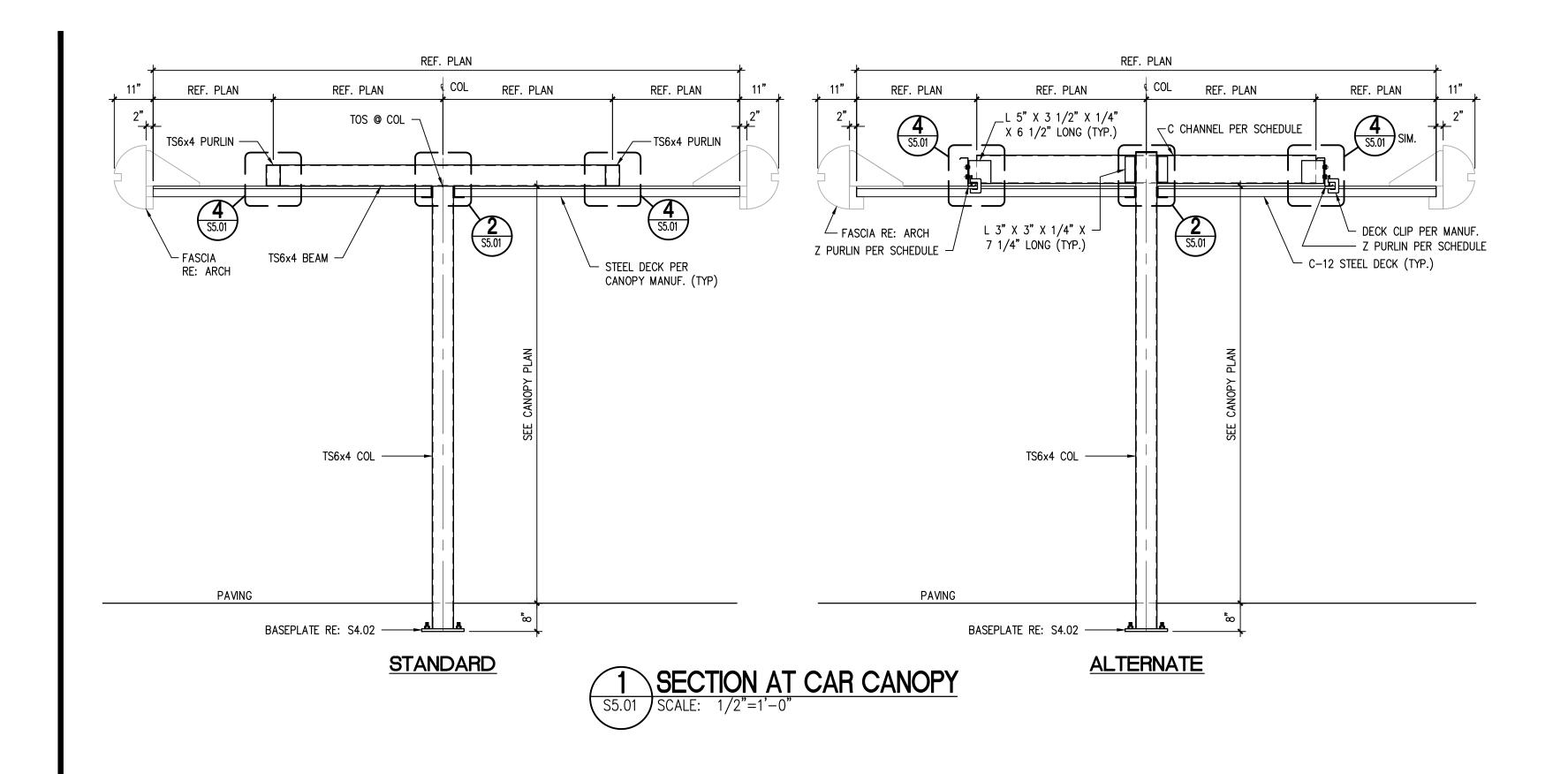
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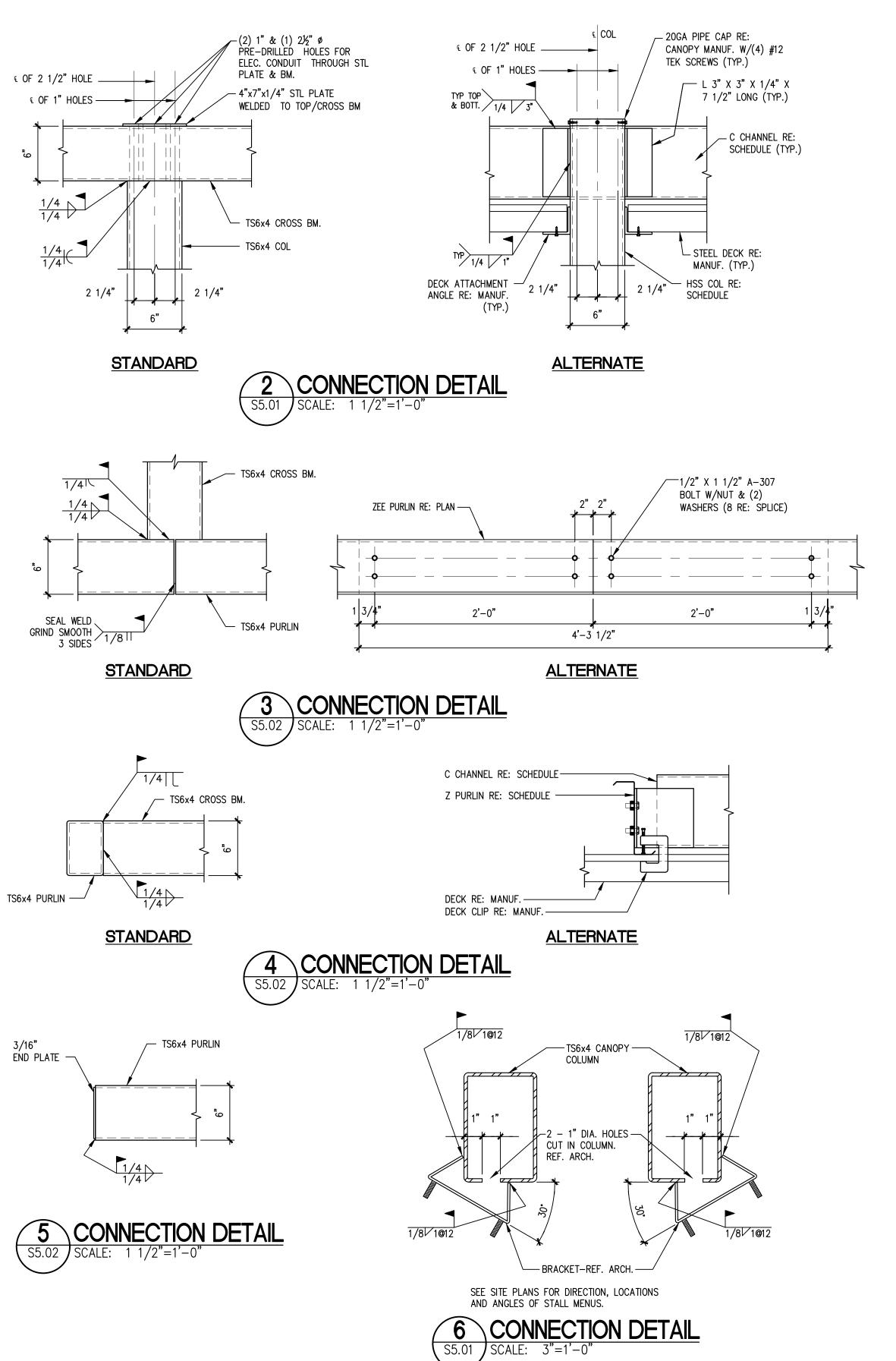
PROJECT LOCATION:

KOKOMO, IN

SHEET NUMBER / TITLE S4.02

CANOPY FOUNDATION PLAN & DETAILS





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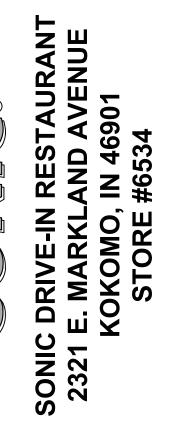
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LIENT:



GEAUX SONIC LLC 1012 SOUTH COOPER STREET GREENTOWN, IN 46901

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